

NOTE: FOR PIGGYBACK  
FASTENING SEE PAGE 14  
OF THE JOBSITE PACKAGE

**Reactions Over 5000 Lbs and Uplifts  
Over 1000 Lbs are Listed on the Layout**

**Hanger Key**

|   |   |   |
|---|---|---|
| (A)=HUS24<br>***=HUS26<br>(C)=HUS28<br>(D)=HUS28-2<br>(E)=HUS28-2 | (F)=HUGUS28-3<br>(G)=GTU80<br>(H)=THGBH43<br>(I)=THGBH44<br>(J)=THA26 | (K)=SUL26<br>(L)=SUR26<br>(M)=HUS46<br>(N)=THA422 |
|---|---|---|

**↑ ↑ ↑ = Hanger Symbol Denotes  
Truss = Truss Connection.**  
**\*\*\* All Trusses are HUS26  
Unless Otherwise Noted.**

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**Design Criteria**

**WMFRS and COMPONENTS & CLADDING**

Wind Load Type : ASCE 7-10

Building Type : Enclosed

Building Exposure : B

Usage : Cat II Residential 1.0

Bottom Chord Analyzed with 10 PSF Non-Concurrent  
Live Load and 20 PSF Concurrent Live Load on  
Trusses Designed with Storage as Specified on  
Detailed Shop Drawings for Specifics.

| GRAVITY |    |     |     | WIND  |    |     |     |
|---------|----|-----|-----|-------|----|-----|-----|
| TC      | DL | 20  | PSF | TC    | DL | 5   | PSF |
| LC      | DL | 20  | PSF | BC    | DL | 5   | PSF |
| BC      | DL | 10  | PSF | TOTAL | 10 | PSF |     |
| TOTAL   | 50 | PSF |     |       |    |     |     |

**DURATION = 1.25      WIND = 170 MPH**

**SPACING: 24 " O.C. Unless Otherwise Noted.**

**Your Signature Will Acknowledge:**

- 1) Authorization for FABRICATION.
- 2) Verification of All Dimensions, Conditions and Trusses. Trusses will be made in STRICT accordance with this Placement Plan. It is YOUR responsibility to check this plan.
- 3) Erection of trusses per TPI Bulletin BCS-181
- 4) ALL permanent and temporary bracing, is CONTRACTOR'S responsibility.
- 5) Any Valleys or Ceiling drops NOT provided by Truss Plant are to be FIELD FILLED by Contractor.
- 6) Truss Plant supplies only TRUSS to TRUSS Connections.
- 7) NO back charges or crane charges of any kind will be accepted unless SPECIFICALLY AUTHORIZED in writing by Truss Plant Management.
- 8) Hip Jacks & Corner Jacks are DOUBLE beveled @ 45. Jacks requiring an angle other than this are to be cut in field by OTHERS.

**Signed:** \_\_\_\_\_

Return One Approved Placement Plan

**Date:** \_\_\_\_\_ Scheduling will NOT start until RE TURNED!!

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**Revisions**

| # | Date   | Remarks        | Int. |
|---|--------|----------------|------|
| 1 | 7/8/15 | ADD RECESS A/H | BJF  |
|   |        |                |      |
|   |        |                |      |
|   |        |                |      |
|   |        |                |      |
|   |        |                |      |
|   |        |                |      |
|   |        |                |      |
|   |        |                |      |

**Raymond Building Supply Corp**

**North Port Myers**  
 7751 Bayshore Rd.  
 N. Port Myers, FL 33917  
 Tel (239) 731-8300  
 Fax (239) 751-0583

**North Port**  
 Tel (841) 428-1212

**Naples**  
 Tel (239) 348-7272

---

**Job Information**

**RBS# : 12010249M11**

**Builder: D.R. HORTON**

**Owner :**

**County:**

**City :**

**Address:**

**Lot :**

**Block :**

**Sub :**

**Model : 2583 ELEV 'E' EXT LANAI**

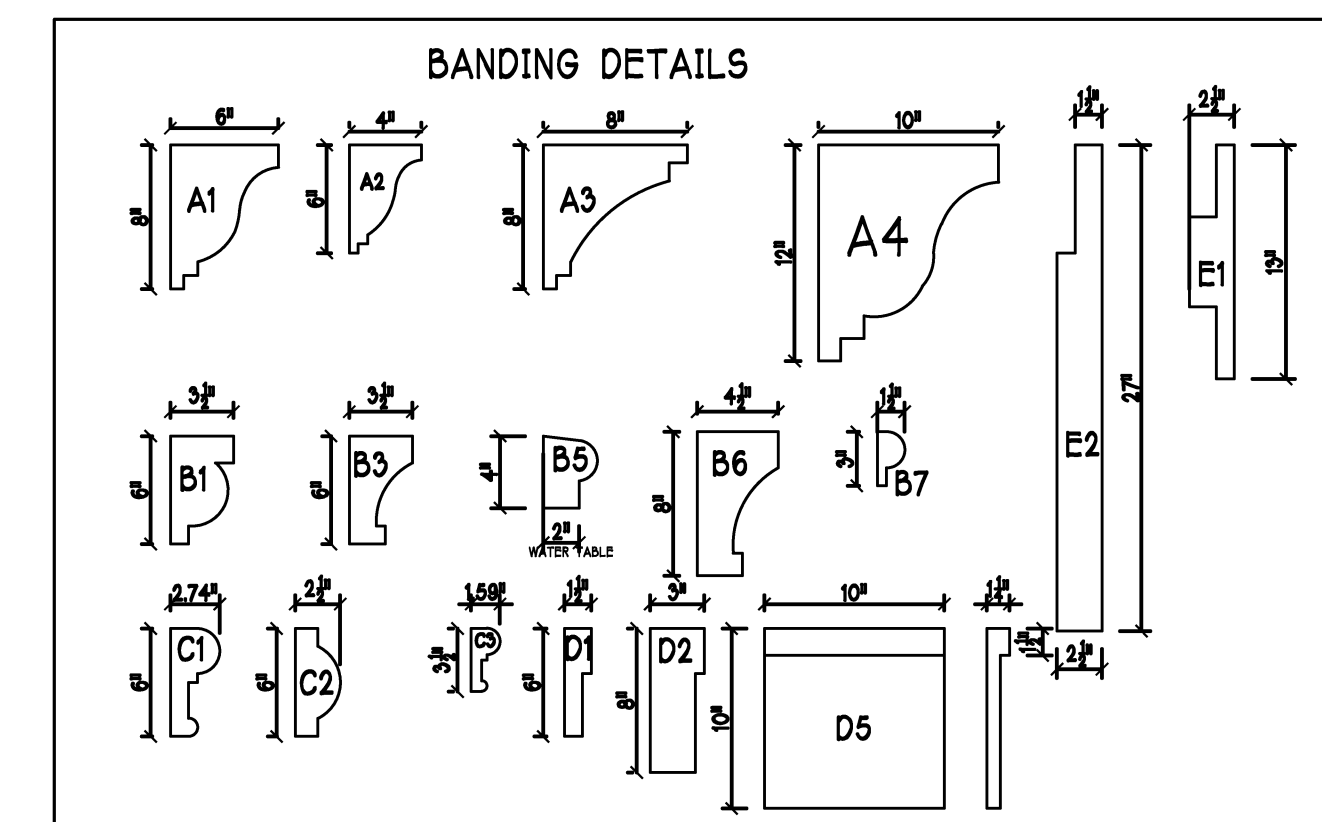
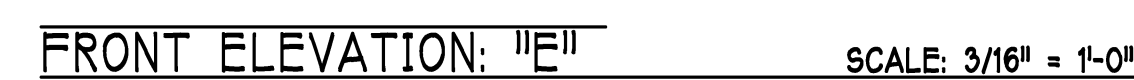
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**Roof Covering: SHINGLE/TILE**

**Scale : 1/4" = 1'-0"**

**Date : May 13, 2014**

**Drawn By: Brad Fischer**



**Gulf Coast Drafting  
& Design**  
Phone (239) 540-1822  
Fax (239) 540-7759

This signature and seal is for work performed by the Structural Engineer of Record (SEOR) related to Structural Engineering only. No work was performed by the SEOR in the areas of Mechanical, Electrical, Plumbing, or other non-Structural Engineering. Safety, accessibility, energy, site work, civil, or geotechnical.

LOT:                      BLOCK:  
SUBDIV: FIDDLER'S CREEK  
ADDRESS: 9479 LAGOMAR CT  
G.C.D. JOB #: 9093      DR.H #: 578600625

|                |           |
|----------------|-----------|
| MODEL:         | UNIT 2583 |
| RESIDENCE FOR: | SPEC      |

DATE: 1-5-16

DRAWN BY:  
CWL

CHECKED BY:  
JWC

REVISÉ:

PLAN: ELEVATION

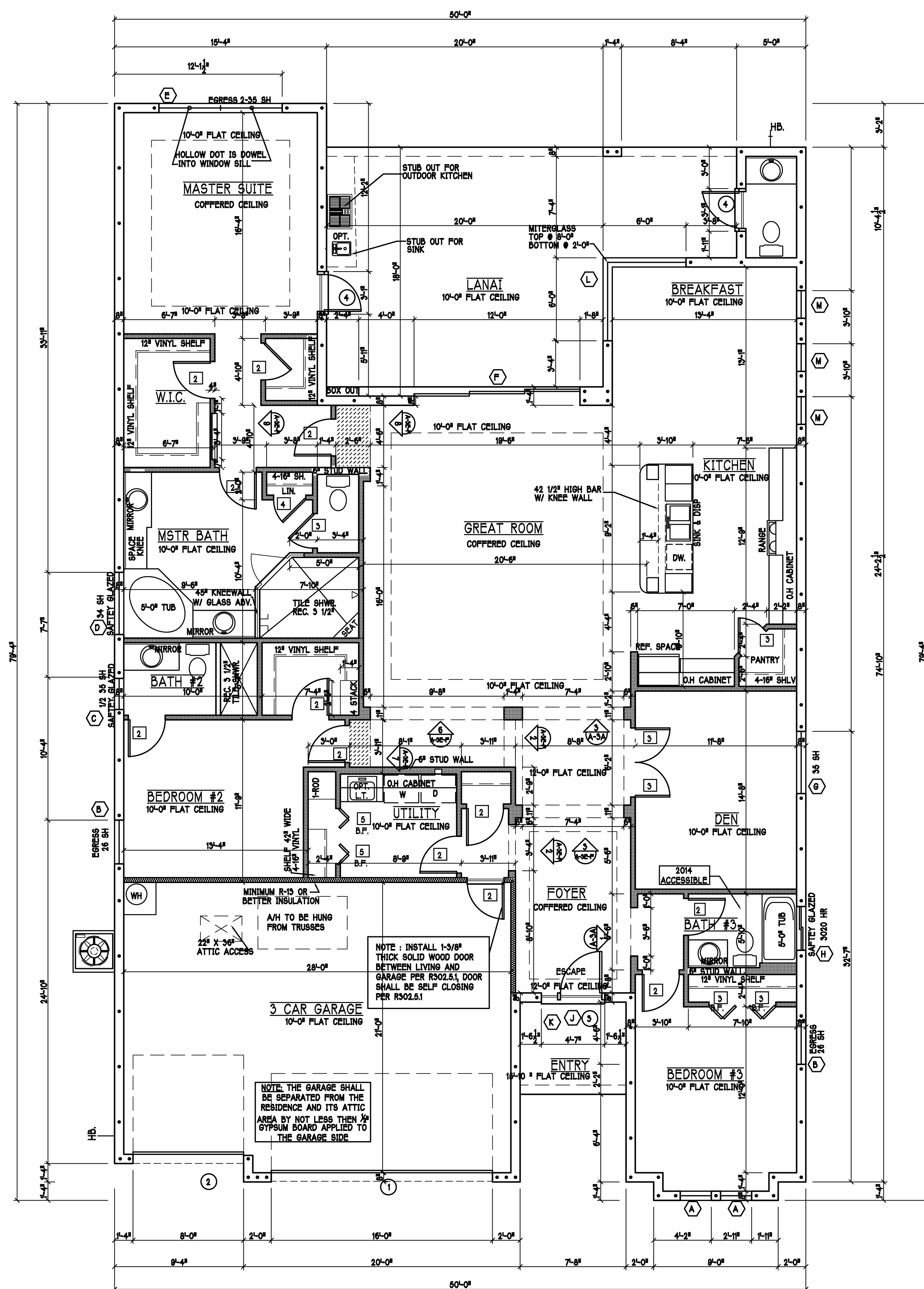
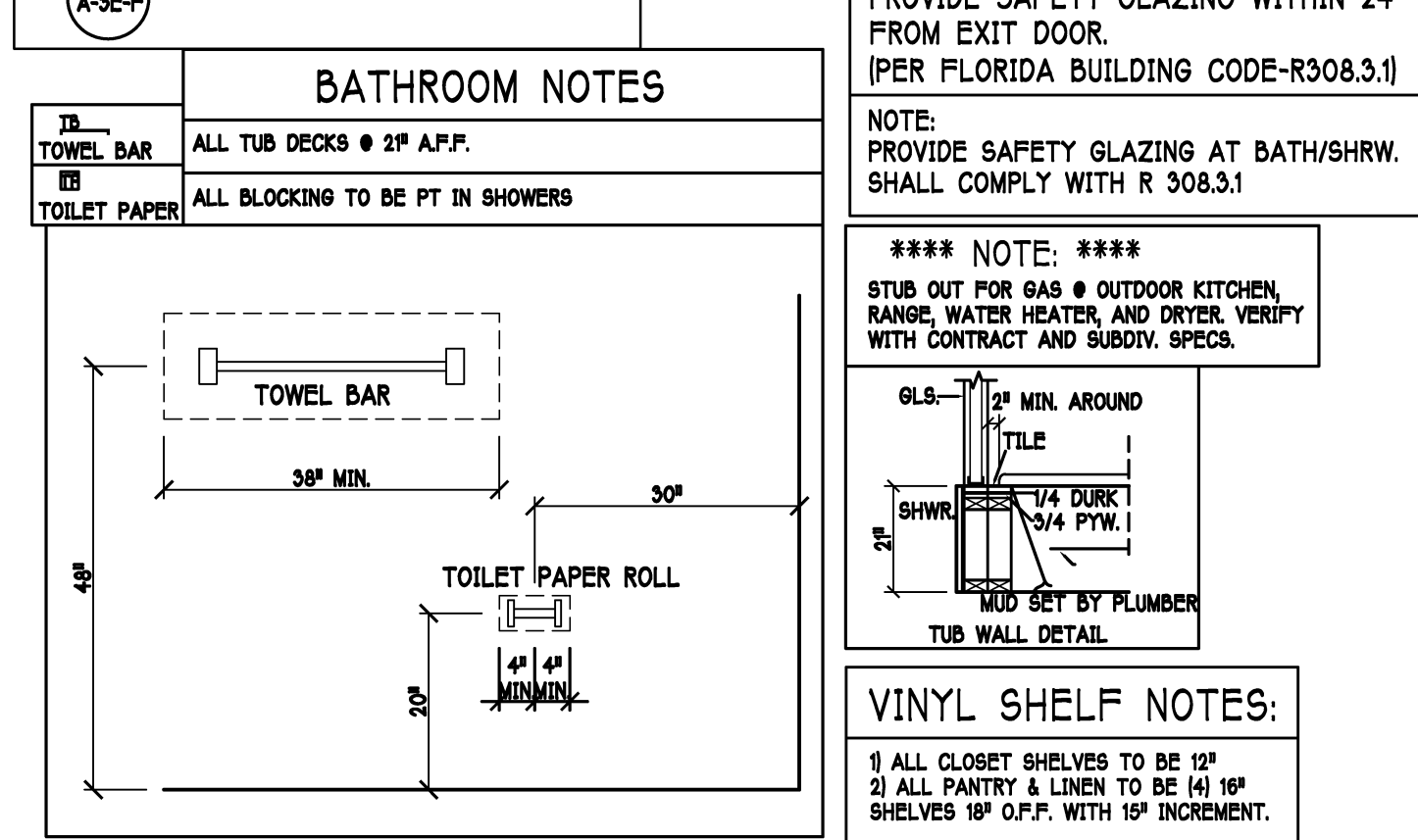
SCALE:  $3/16"=1'-0"$

SHEET#

A-1E

DESIGN IN ACCORDANCE W/ THE 2014 RESIDENTIAL  
FLORIDA BUILDING CODE- 5TH EDITION





FLOOR PLAN: "E&F" SCALE: 3/16" = 1'-0"

| WIND PRESSURES PER ASCE7-09, 170 MPH EXPOSURE B, AND CONVERTED TO ALLOWABLE STRESS DESIGN USING 0.6W LOAD FACTOR<br>V=132 MPH |                |                     |            |             |      |               |                              |     |
|---|----------------|---------------------|------------|-------------|------|---------------|------------------------------|-----|
| D.R   | HORZ. SECTION  | PRODUCT DESCRIPTION | DOOR WIDTH | DOOR HEIGHT | ZONE | WIND PRESSURE | WIND-BORNE DEBRIS PROTECTION | QTY |
| ①   | OVERHEAD       | GARAGE DOOR         | 192        | 96          | 4A-5 | +26.4/-28.4   |                              | 1   |
| ②   | OVERHEAD       | GARAGE DOOR         | 96         | 96          | 4A-5 | +27.8/-31.2   |                              | 1   |
| ③   | 3080 ENTRY DR. | DISTINCTION         | 56         | 96          | 4    | +51.2/-35.8   |                              | 1   |
| ④   | 2880 ENTRY DR. | DISTINCTION         | 56         | 96          | 4    | +51.2/-35.8   |                              | 1   |

| WIND PRESSURES PER ASCET-10, TO MPH, EXPOSURE B, AND CONVERTED TO ALLOWABLE STRESS DESIGN PRESSURES USING 0.6W LOAD FACTOR<br>Voeckel's MPH |                         |                     |      |               |                              |     |
|---|-------------------------|---------------------|------|---------------|------------------------------|-----|
| D R HORTON  |                         |                     |      |               |                              |     |
| MARK  | SIZE CODE               | PRODUCT DESCRIPTION | ZONE | WIND PRESSURE | WIND-BORNE DEDRIS PROTECTION | QTY |
| (A)   | V/2 36 SH               |                     | 4    | +31/2/-35.8   | IMPACT                       | 2   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (B)   | 26 SH                   |                     | 4    | +31/2/-35.8   | IMPACT                       | 2   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (C)   | V/2 35 SH               |                     | 4    | +31/2/-35.8   | IMPACT                       | 2   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (D)   | 34 SH                   |                     | 4    | +31/2/-35.8   | IMPACT                       | 1   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (E)   | 2-35 SH                 |                     | 4    | +31/2/-35.8   | IMPACT                       | 1   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (P)   | 3-4080PKT<br>SL GL DR   | SL GL DOOR          | 4    | +27.6/-30.1   | IMPACT                       | 1   |
|   |                         |                     | 5    | +27.5/-34.5   |                              |     |
| (B)   | 35 SH                   |                     | 4    | +31/2/-35.8   | IMPACT                       | 1   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (H)   | 2030 HR                 |                     | 4    | +31/2/-35.8   | IMPACT                       | 1   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (J)   | 85" X 243"<br>FIXED GL  | 10° AND 20° ONLY    | 4    | +31/2/-35.8   | IMPACT                       | 1   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (K)   | 86" X 138"<br>SIDE LITE |                     | 4    | +31/2/-35.8   | IMPACT                       | 1   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (L)   | 72" X 72"               | MITERED GLASS       | 4    | +31/2/-35.8   | IMPACT                       | 1   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |
| (W)   | 24" X 24"<br>FIXED GL   |                     | 4    | +31/2/-35.8   | IMPACT                       | 3   |
|   |                         |                     | 5    | +31/2/-41.8   |                              |     |

15

- # PLAN NOTES
- 1) VERIFY ALL ROUGH OPENING DIMENSIONS FOR ALL WINDOWS AND DOORS
  - 2) PROVIDE SPACER GLAZING WITHIN 2"4" FROM EXIST PER FLORIDA BUILDING CODE R 308.5.1
  - 3) PROVIDE SPACER GLAZING AT BATH / SHOWER / PER FLORIDA BUILDING CODE R 308.5.1
  - 4) NON BEARING INTERIOR FRAME WALLS SHALL BE FINISHED W/ WOOD OR VINYL STUDS. SPACING SHALL NOT EXCEED 24" O.C. (NON BEARING WALLS ONLY)
  - 5) PROVIDE DEAD WOOD IN ATTIC FOR OVERHEAD GARAGE DOOR HARDWARE
  - 6) 2X6 KITCHEN KNEE WALL 5/4" TO TOP
  - 7) WHERE DRYWALL CEILING IS APPLIED TO TRUSSES AT 24" O.C. USE 5/8" DRYWALL OR 1/2" SAG RESISTANT PER SEC. 702.3.5
  - 8) THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE BY A ATTIC BY NOT LESS THAN 1/2" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. GARAGES BENEATH HABITABLE ROOMS SHALL BE SEPARATED WITH NOT LESS THAN 5/8" TYPE "X" GYPSUM BOARD OR EQUIVALENT. WHERE THE SEPARATION IS A FLOOR - CEILING ASSEMBLY THE STRUCTURE SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED BY NOT LESS THAN 1/2" GYPSUM BOARD OR EQUIVALENT
  - 9) INSTALL 1 5/8" THICK SOLID WOOD FLOOR BETWEEN LIVING AND GARAGE PER FLORIDA BUILDING CODE R02.5.1 HEADER @ 83" DOOR SHALL BE SELF CLOSING R02.5.1
  - 10) ALL WINDOWS INSTALL 7/8" ABOVE GRADE MUST COME WITH R 602.8 MIN 24" SILL HEIGHT OR PROVIDED WITH AN APPROVED WINDOW FALL PREVENTION DEVICE
  - 11) STUB OUT FOR GAS @ OUTDOOR KITCHEN, RANGE, WATER HEATER AND DRYER. VERIFY WITH CONTRACTOR AND GAS SUPPLIER. A SEPARATE PERMIT IS REQUIRED FOR GAS PIPING.
  - 12) ALL CLOSET SHELVES TO BE 12". ALL PANTRY & LINEN TO BE 14-16" SHELVES 18" O.F.F. WITH 16" DEPTH

| CABINET BACKING |  |                 |
|-----------------|--|-----------------|
| KITCHEN         | UPPER TOP @ 54", 54" & 86"<br>MICROWAVE @ 102" | BASE TOP @ 35"  |
| MASTER BATH     | UPPER  | BASE- TOP @ 35" |
| GUEST BATH      | UPPER  | BASE- TOP @ 31" |
| LAUNDRY RM.     | UPPER TOP @ 84"                                | BASE            |

- BATHROOM NOTES

- ALL TUB DECKS • 2
- <sup>nd</sup>
- A.F.F.

- ALL BLOCKING TO BE PT IN SHOWERS

| DOOR HEADERS |               |                |
|--------------|---------------|----------------|
| 6'-8" BIFOLD | HEADER HEIGHT | 82" A.F.F.     |
| 6'-8" SWING  | HEADER HEIGHT | 82 1/2" A.F.F. |
| 8'-0" SWING  | HEADER HEIGHT | 88 1/2" A.F.F. |

| SQUARE FOOTAGE   |       |
|------------------|-------|
| LIVING AREA      | 2,583 |
| LANAI AREA       | 477   |
| GARAGE AREA      | 618   |
| ENTRY AREA       | 51    |
| TOTAL AREA 3,729 |       |

DESIGN IN ACCORDANCE W/ THE 2014 RESIDENTIAL  
FLORIDA BUILDING CODE- 5TH EDITION

**D·R·HORTON** • PHILADELPHIA  
*America's Builder*

**Gulf Coast Drafting  
& Design**  
Phone (239) 540-1822  
Fax (239) 540-7759

|   |   |
|---|---|
| <p>This signature and seal is for work performed by the Structural Engineer of Record (SER) related to Structural Engineering only. No work was performed by the SER in the areas of Mechanical, Electrical, Plumbing, Fire Protection, Civil, Environmental, Life Safety, Accessibility, Energy, Site Work, Civil or Geotechnical.</p> | <p>STRUCTURAL ENGINEERING</p>   |
| <p><b>STRUCTURAL SYSTEMS</b></p>  | <p>1634 SE 47TH ST. SUITE #2<br/>CAPE CORRAL, FL 33504<br/>(813) 549-4554</p> |

LOT: BLOCK :  
 SUBDIV: FIDDLER'S CREEK  
 ADDRESS: 9479 LAGOMAR CT  
 C D IOP # : 0003 D P U # : 570600625

|                |           |
|----------------|-----------|
| MODEL:         | UNIT 2583 |
| RESIDENCE FOR: | SPEC      |

DATE: 1-5-16

DRAWN BY:

[illegible]

CHECKED BY

[illegible]

REVISÉ:

PLAN

PLAN:

SCALE:

3

SHEET#

**1**

A-

11

SOFFIT VENTS AROUND FULL PERIMETER  
NO FIRE RATED SOFFIT

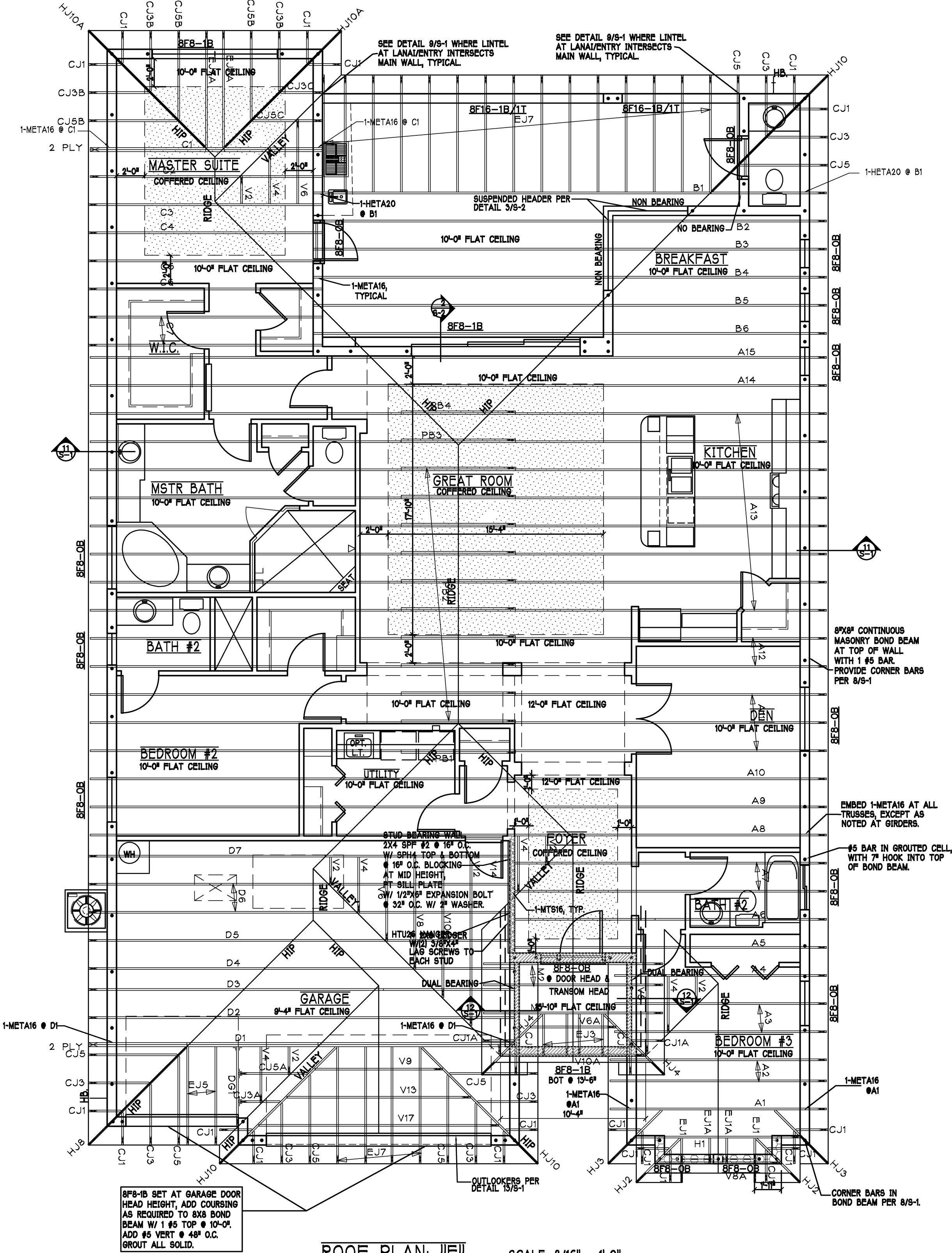
| ATTIC VENTILATION                                    |                |  |                        |  |                 |
|--|----------------|--|------------------------|--|-----------------|
| verify venting requirements with energy calculations |                | WITHOUT OFF RIDGE VENTS  |                        | WITH OFF RIDGE VENTS   |                 |
| ATTIC AREA (FBC R806)                                |                | VENTILATION REQUIRED (ATTIC AREA 1/150)  |                        | VENTILATION REQUIRED (ATTIC AREA 1/900 INSTALL PER FBC R806.2 MINIMUM AREA REQUIREMENTS) |                 |
| mark   | square footage | soffit vents   | MIN AIR FLOW OF SOFFIT | total ventilation  | off ridge vents |
| ①  | 3452 SQ. FT.   | 23.01 SQ. FT.  | 5.7%                   | O.R.V. NOT USED  |                 |
|  |                | ATTIC VENTILATION CALCULATION:<br>attic sq. ft. / 150 = vented sq. ft.           |                        | ATTIC VENTILATION CALCULATION:<br>attic sq. ft. / 900 = vented sq. ft.                   |                 |
|  |                | OFF RIDGE EXHAUST VENT SIZES<br>(AREA NET FREE SQUARE FEET)<br>SCALE: 1/4"=1'-0" |                        |  |                 |
|  |                | 8'-0" BASE<br>146 SQ. FT. FREE AREA  |                        | 30" BASE<br>1 SQ. FT. FREE AREA  |                 |
|  |                | 2'-0" BASE<br>38 SQ. FT. FREE AREA   |                        |  |                 |

| TRUSS STRAPPING TO MASONRY      |                 |                        |  |
|---------------------------------|-----------------|------------------------|--|
| MAX TRUSS UPLIFT @ 24" OC (LBS) | CONNECTOR       | FASTENER               |  |
| 1450                            | (1)META16 TO 40 | 9-10dx1 1/2" EMBED 4"  |  |
| 1810                            | (1)META16 TO 40 | 10-10dx1 1/2" EMBED 4" |  |
| 2235                            | (1)HMET16 TO 40 | 12-10dx1 1/2" EMBED 4" |  |
| 1985 (1 PLY)                    | (2)META12 TO 40 | 12-10dx1 1/2" EMBED 4" |  |
| 1900 (2 PLY)                    | (2)META12 TO 40 | 14-16d, EMBED 4"       |  |
| 2500 (2 PLY)                    | (2)META12 TO 40 | 14-16d, EMBED 4"       |  |
| 2500 (2 PLY)                    | (2)HMET12 TO 22 | 14-16d, EMBED 4"       |  |

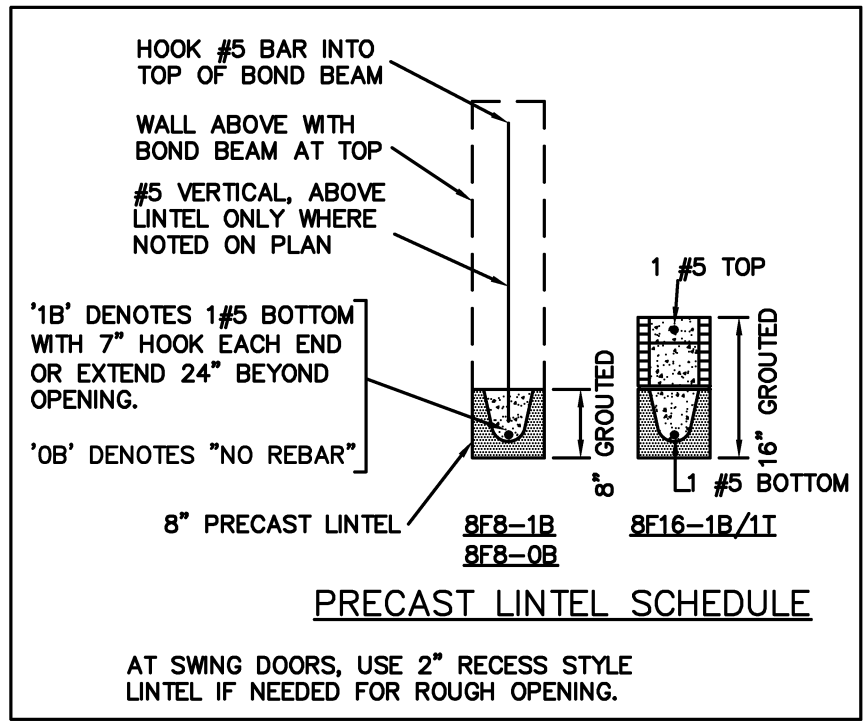
NOTES:  
1) PROVIDE A STRAP FROM THE ABOVE LIST AT EACH ROOF TRUSS BEARING POINT, BASED ON THE TRUSS UPLIFT VALUES IN THE SIGNED AND SEALED TRUSS DESIGN PACKAGE AND SUITABLE FOR THE GEOMETRY. EMBED STRAP ON @ OF WALL.  
2) CONNECTORS ARE SIMPSON STRONG TIE. ALL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH SIMPSON PRINTED INSTRUCTIONS. SUBSTITUTIONS MUST BE APPROVED IN WRITING BY THE ENGINEER OF RECORD.  
3) WHERE EMBEDDED STRAPS ARE MISSING, OR MIS-LOCATED, INSTALL RETROFIT STRAP PER 10/S-1.

REV 2

- PLAN NOTES:  
1) ROOF TRUSS BEARING ELEVATION VARIES, SEE LEGEND.  
2) ROOF FRAMING SHALL BE WOOD TRUSSES DESIGNED BY A DELEGATED TRUSS ENGINEER PER DESIGN CRITERIA ON SHEET S-1.  
3) PROVIDE STRAPPING AT TRUSSES PER NOTES ON THIS SHEET.  
4) FOR NAILING OF ROOF DECK, SEE 1 AND 2 ON S-1.  
5) [8F8-1B] etc., DENOTES PRECAST LINTEL ABOVE DOOR/WINDOW OPENING PER SCHEDULE. THIS SHEET.  
6) AT TRUSS BEARING, PROVIDE 6x8 MASONRY BOND BEAM W/ 1 #5 CONTINUOUS, SEE DETAIL 2/A-6.



ROOF PLAN: 11E11 SCALE: 3/16" = 1'-0"



TRUSS BEARING CONDITIONS AND STRAPPING IS BASED ON TRUSS LAYOUT PREPARED BY RAYMOND JOB # 12010249M11, DATED: 05/13/14 REVISION DATE: 07/08/15

### BEARING HEIGHT

- BEARING @ 10'-0" A.F.F.
- BEARING @ 11'-0" A.F.F.
- INTERIOR BEARING @ 12'-0" A.F.F.
- BEARING @ 13'-0" A.F.F.

DESIGN IN ACCORDANCE W/ THE 2014 RESIDENTIAL FLORIDA BUILDING CODE- 5TH EDITION

**D.R. HOHON**  
*America's Builder*

Gulf Coast Drafting & Design  
Phone (239) 540-1822  
Fax (239) 540-7759

STRUCTURAL ENGINEERING  
**STRUCTURAL SYSTEMS**  
1634 SE 47th ST, SUITE # 200  
CAPE CORAL, FL 33904  
(239) 540-4554  
CEN 8825

LOT: BLOCK:  
SUBDIV: FIDDLER'S CREEK  
UNIT 2583  
ADDRESS: 9479 LAGOMAR CT  
G.C.D. JOB #: 9093 D.R.H.#: 578600625

MODEL: UNIT 2583  
RESIDENCE FOR: SPEC

DATE: 1-5-16  
DRAWN BY: CWL  
CHECKED BY: JWC  
REVISED:  
PLAN: ROOF  
SCALE: 3/16"=1'-0"  
SHEET# A-4E

SOFFIT VENTS AROUND FULL PERIMETER  
NO FIRE RATED SOFFIT

| ATTIC VENTILATION                                    |                |  |                        |  |                 |                        |
|--|----------------|--|------------------------|--|-----------------|------------------------|
| verify venting requirements with energy calculations |                | WITHOUT OFF RIDGE VENTS  |                        | WITH OFF RIDGE VENTS   |                 |                        |
| ATTIC AREA (FBC R806)                                |                | VENTILATION REQUIRED (ATTIC AREA 1/150)                                |                        | VENTILATION REQUIRED (ATTIC AREA 1/500 INSTALL PER FBC R806.2 MINIMUM AREA REQUIREMENTS) |                 |                        |
| mark   | square footage | soffit vents   | MIN AIR FLOW OF SOFFIT | total ventilation  | off ridge vents | MIN AIR FLOW OF SOFFIT |
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|  |                | ATTIC VENTILATION CALCULATION:<br>attic sq. ft. / 150 = vented sq. ft. |                        | ATTIC VENTILATION CALCULATION:<br>attic sq. ft. / 500 = vented sq. ft.                   |                 |                        |

6'-0" BASE

145 SQ. FT. FREE AREA

2'-0" BASE

1 SQ. FT. FREE AREA

36" BASE

36 SQ. FT. FREE AREA

OFF RIDGE EXHAUST VENT SIZES  
(AREA NET FREE SQUARE FEET)  
SCALE: 1/4"=1'-0"

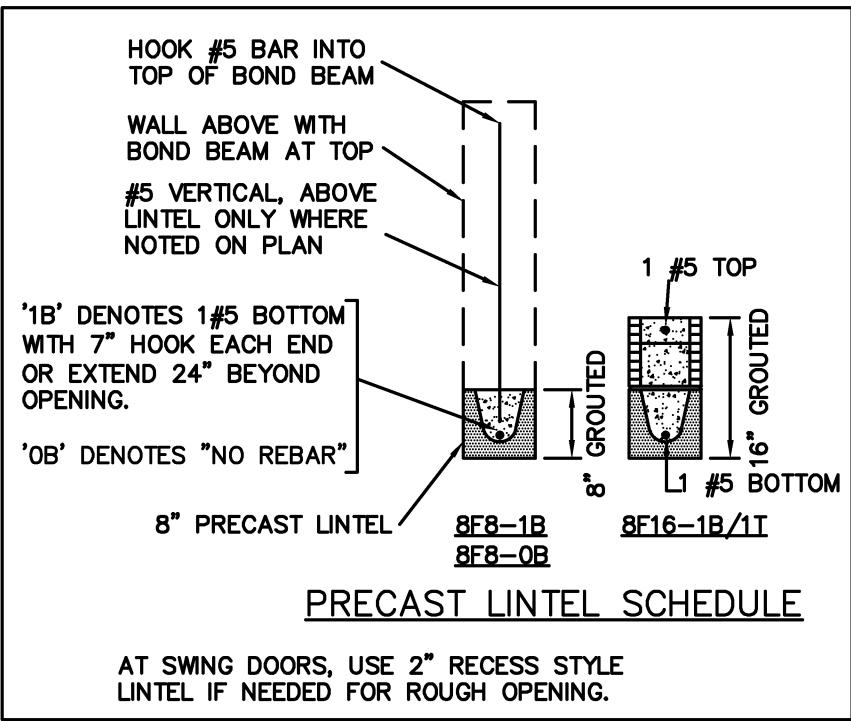
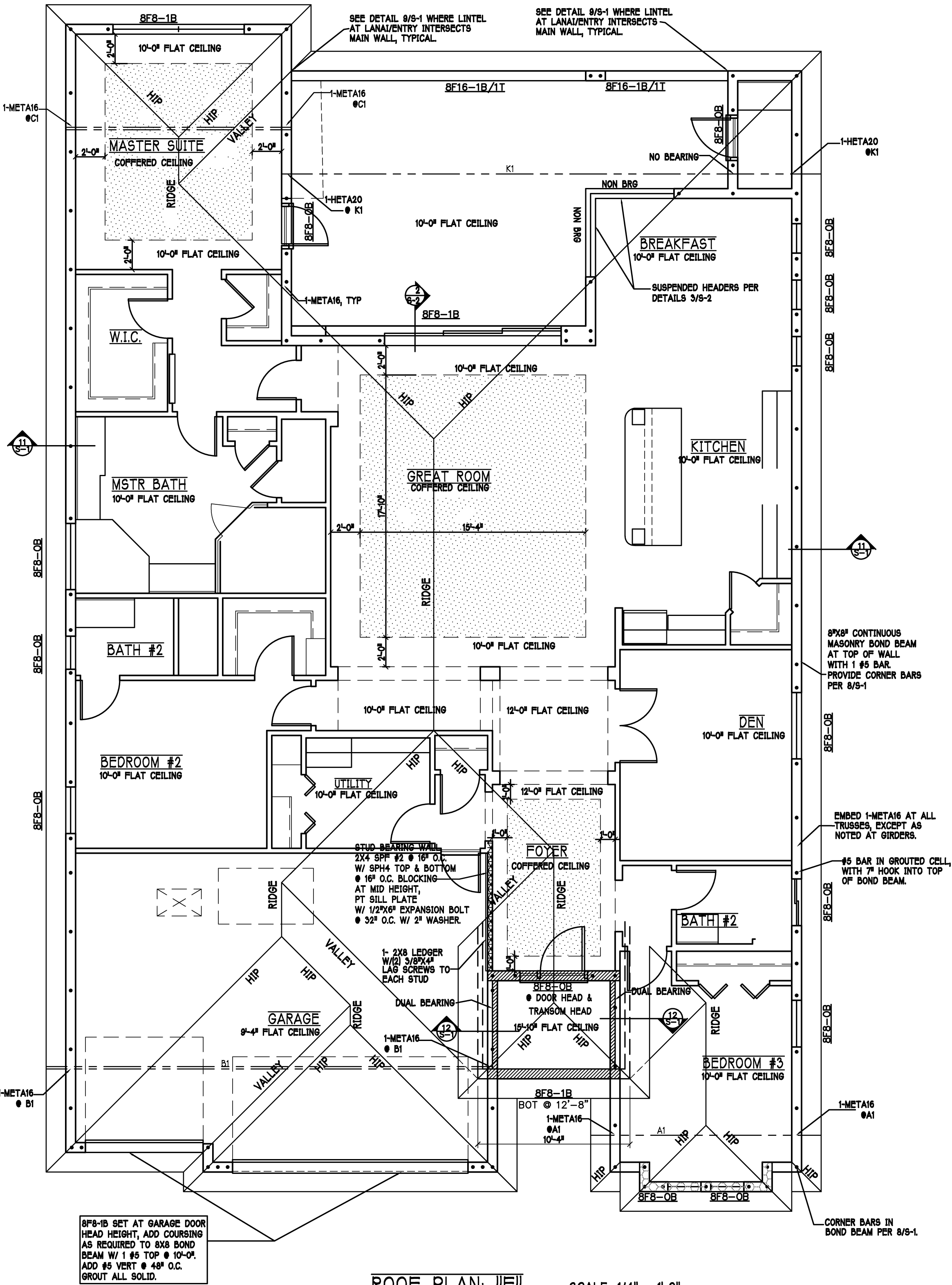
| TRUSS STRAPPING TO MASONRY   |                                    |                 |                        |
|--|------------------------------------|-----------------|------------------------|
| INSTALL<br>META16 AT<br>ALL<br>TRUSSES<br>TO 1450 lb<br>UPLIFT.<br>FOR HIGHER<br>UPLIFTS,<br>SEE NOTES<br>ON PLAN. | MAX TRUSS UPLIFT<br>● 24" OC (LBS) | CONNECTOR       | FASTENER               |
|  | 1450                               | (1)META16 TO 40 | 9-10dx1 1/2" EMBED 4"  |
|  | 1810                               | (1)HETA16 TO 40 | 10-10dx1 1/2" EMBED 4" |
|  | 2235                               | (1)HETA16 TO 40 | 12-10dx1 1/2" EMBED 4" |
|  | 1985 (1 PLY)                       | (2)META12 TO 40 | 12-10dx1 1/2" EMBED 4" |
|  | 1900 (2 PLY)                       | (2)META12 TO 40 | 14-16d, EMBED 4"       |
|  | 2500 (2 PLY)                       | (2)HETA12 TO 40 | 14-16d, EMBED 4"       |
|  | 2500 (2 PLY)                       | (2)HETA12 TO 22 | 14-16d, EMBED 4"       |

NOTES:

- 1) PROVIDE A STRAP FROM THE ABOVE LIST AT EACH ROOF TRUSS BEARING POINT, BASED ON THE TRUSS UPLIFT VALUES IN THE SIGNED AND SEALED TRUSS DESIGN PACKAGE AND SUITABLE FOR THE GEOMETRY. EMBED STRAP ON 6" OF WALL.
- 2) CONNECTORS ARE SIMPSON STRONG TIE. ALL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH SIMPSON PRINTED INSTRUCTIONS. SUBSTITUTIONS MUST BE APPROVED IN WRITING BY THE ENGINEER OF RECORD.
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REV 2

- PLAN NOTES:  
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4) FOR NAILING OF ROOF DECK, SEE 1 AND 2 ON S-1.  
5) (SEB-1B) etc. DENOTES PRECAST LINTEL ABOVE DOOR/WINDOW OPENING PER SCHEDULE THIS SHEET.  
6) AT TRUSS BEARING, PROVIDE 8x8 MASONRY BOND BEAM W/ 1 #5 TOP @ 10'-0" ADD #5 VERT @ 48" O.C. (GROUT ALL SOLID.



TRUSS BEARING CONDITIONS AND STRAPPING IS BASED ON TRUSS LAYOUT PREPARED BY RAYMOND BUILDING SUPPLY JOB #: 151214015, DATED: 12/01/15, REVISED: NONE

| BEARING HEIGHT |                                    |
|----------------|------------------------------------|
|                | • BEARING @ 10'-0" A.F.F.          |
|                | • BEARING @ 11'-0" A.F.F.          |
|                | • INTERIOR BEARING @ 12'-0" A.F.F. |
|                | • BEARING @ 15'-10" A.F.F.         |

DESIGN IN ACCORDANCE W/ THE 2014 RESIDENTIAL FLORIDA BUILDING CODE- 5TH EDITION

**D.R.HOHN**  
*America's Builder*

Gulf Coast Drafting  
& Design  
Phone (239) 540-1822  
Fax (239) 540-7759

STRUCTURAL ENGINEERING  
**STRUCTURAL SYSTEMS**  
OF NORTH FLORIDA  
1506 S.E. 47th ST. SUITE #3  
FORT MYERS, FL 33901  
(239) 549-4554  
CM 8869

LOT: BLOCK :  
SUBDIV: FIDDLER'S CREEK  
ADDRESS: 9479 LAGOMAR CT  
G.C.D. JOB # : 9093 D.R.H.# : 578600625  
MODEL: UNIT 2583  
RESIDENCE FOR: SPEC

DATE: 1-5-16  
DRAWN BY: CWL  
CHECKED BY: JWC  
REVISED:  
PLAN: ROOF  
SCALE: 3/16"=1'-0"  
SHEET#

A-4 F



10. LANAI CEILINGS & COVERED ENTRY CEILINGS  
1X4 STRIPPING @ 16" O.C. FASTENED WITH 2-8d NAILS TO EACH TRUSS. 5/8"  
EXTERIOR GYPBOARD CEILING FASTENED WITH 8d NAILS OR 1-5/8" DRYWALL  
SCREWS @ 6" oc. EDGE AND FIELD.

WOOD FRAMED OPENING- ALL DOORS AND WINDOWS SHALL BE INSTALLED ACCORDING TO THE PUBLISHED MANUFACTURES LITERATURE OF THE ASSEMBLY BEING INSTALLED TO THE ROUGH SUBSTRATE OPENING. SHIMS SHALL BE MADE OF MATERIALS CAPABLE OF RESISTING THE APPLIED LOADS AND SHALL BE LOCATED NEAR EACH FRAME FASTENER TO MINIMIZE DISTORTION OF THE FRAME AS THE FASTENERS ARE TIGHTENED.

DRIP EDGE  
DRIP EDGE SHALL BE PROVIDED AT ALL EAVES AND GABLES OF SHINGLE ROOFS, LAPPED A MINIMUM OF 3" @ JOINTS. THE OUTSIDE EDGE SHALL EXTEND A MINIMUM OF 2" BELOW SHEATHING AND THE INSIDE EDGE SHALL EXTEND BACK A MINIMUM OF 2". DRIP EDGE SHALL BE FASTENED AT NO MORE THAN 4" CENTERS. THERE SHALL BE A MINIMUM OF 4" WIDTH OF ROOF CEMENT INSTALLED OVER THE DRIP EDGE FLANGE.

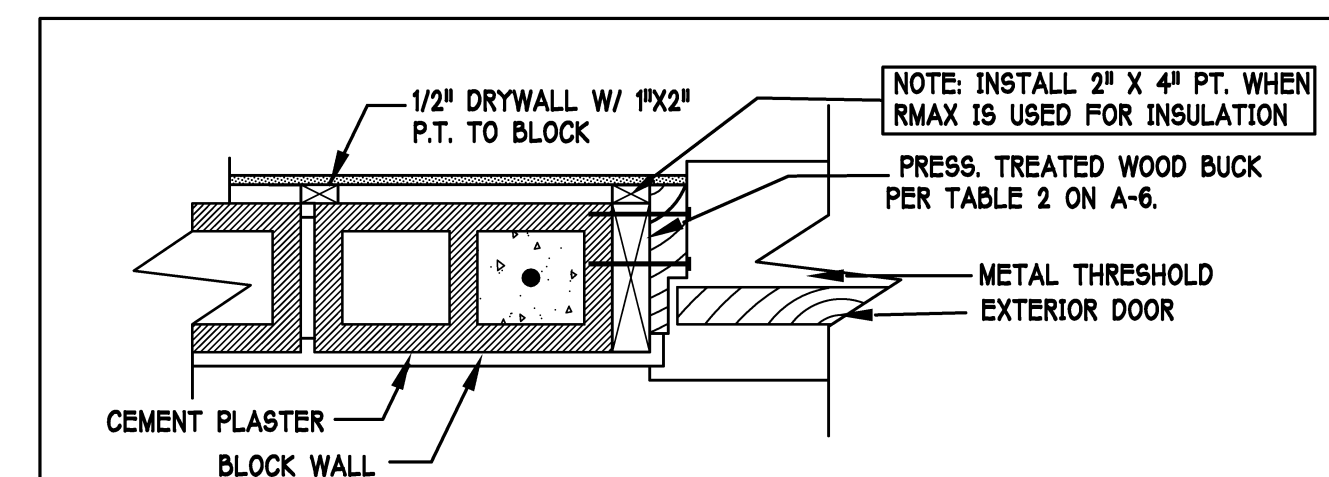
15. SPACE FRAMING OF ARCHES UNDER TIE BEAM SHALL BE  
FILL IN FRAME UNLESS NOTED OR CONSTRUCTED OTHERWISE

The nail component of plastic cap nails shall meet or exceed the requirements of ASTM A 641, Class I, or equal, and shall be corrosion resistant by coating electro galvanization, mechanical galvanization, hot dipped galvanization or shall be made of stainless steel, non ferrous metal.

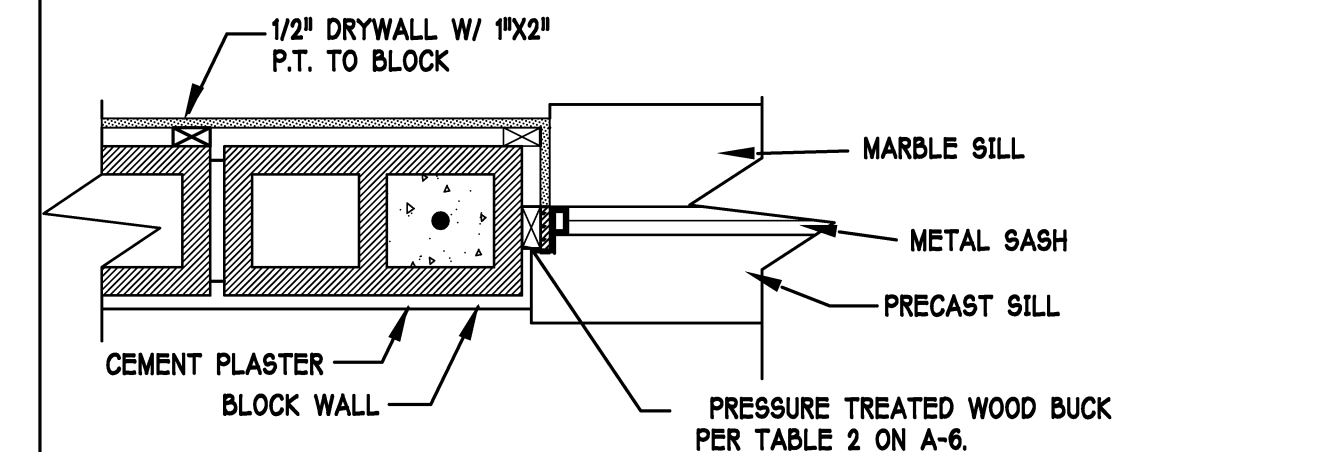
1. TILE PLACEMENT AND SPACING;
2. ATTACHMENT SYSTEM NECESSARY TO COMPLY WITH CURRENT WIND CODE,
  - A. AMOUNT AND PLACEMENT OF MORTAR
  - B. AMOUNT AND PLACEMENT OF ADHESIVE;
  - C. TYPE, NUMBER, SIZE, AND LENGTH OF FASTENERS AND CLIPS.
3. UNDERLAYMENT
4. SLOPE REQUIREMENT.

A.P.A. RATED STURDI-FLOOR, EXPOSURE 1, TONGUE & GROOVE EDGES  
SPAN RATING 48/24 OR BETTER, GLUE AND NAIL W/  
10d COMMON @ 6" O.C. EDGE AND FIELD.

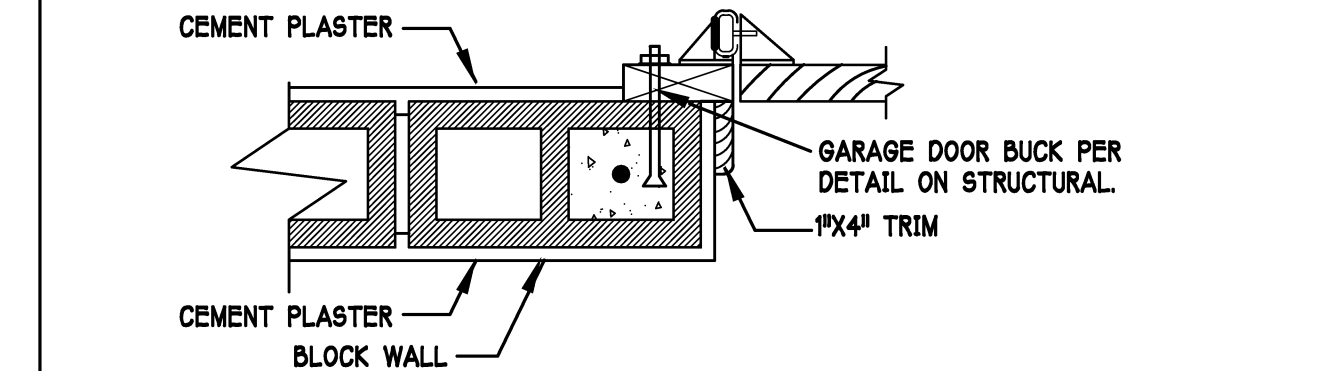
SHALL BE 7/16" THICK ZIP SYSTEM WALL SHEATHING\* MANUFACTURED BY HUBER ENGINEERED WOODS LLC. INSTALL PANELS WITH A 1/8" GAP BETWEEN EDGES AND FASTEN WITH 8d COMMON NAILS @ 6" O.C. EDGE AND FIELD. IF PANELS ARE INSTALLED HORIZONTALLY, BLOCKING SHALL BE INSTALLED BEHIND PANEL JOINTS. ALL SEAMS IN THE SHEATHING SHALL BE SEALED WITH THE ZIP SYSTEM SELF ADHERING SEAM TAPE USING THE ZIP SYSTEM APPLICATOR GUN. THE USUAL TYVEK HOUSE WRAP IS NOT REQUIRED.



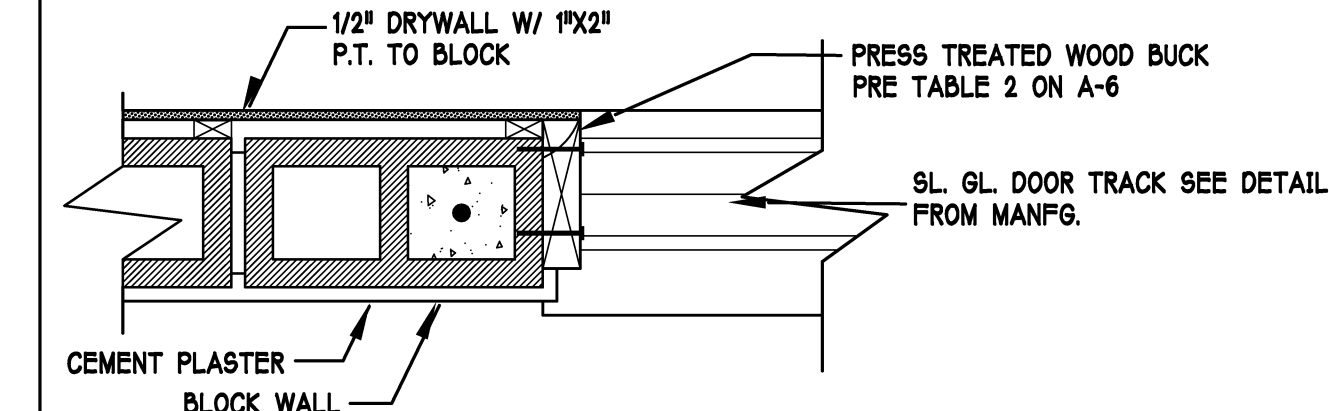
DOOR JAM TO BLOCK DETAIL SCALE 1" = 1'-0"



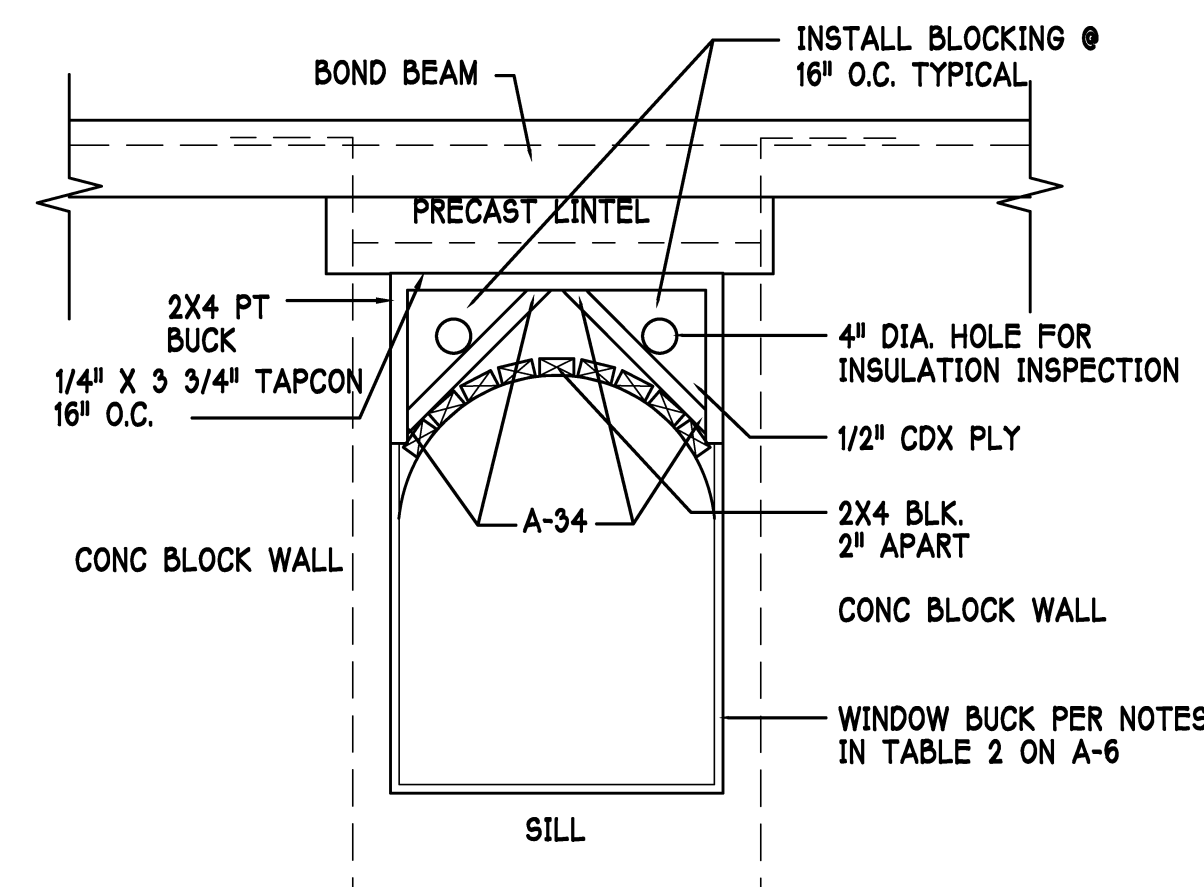
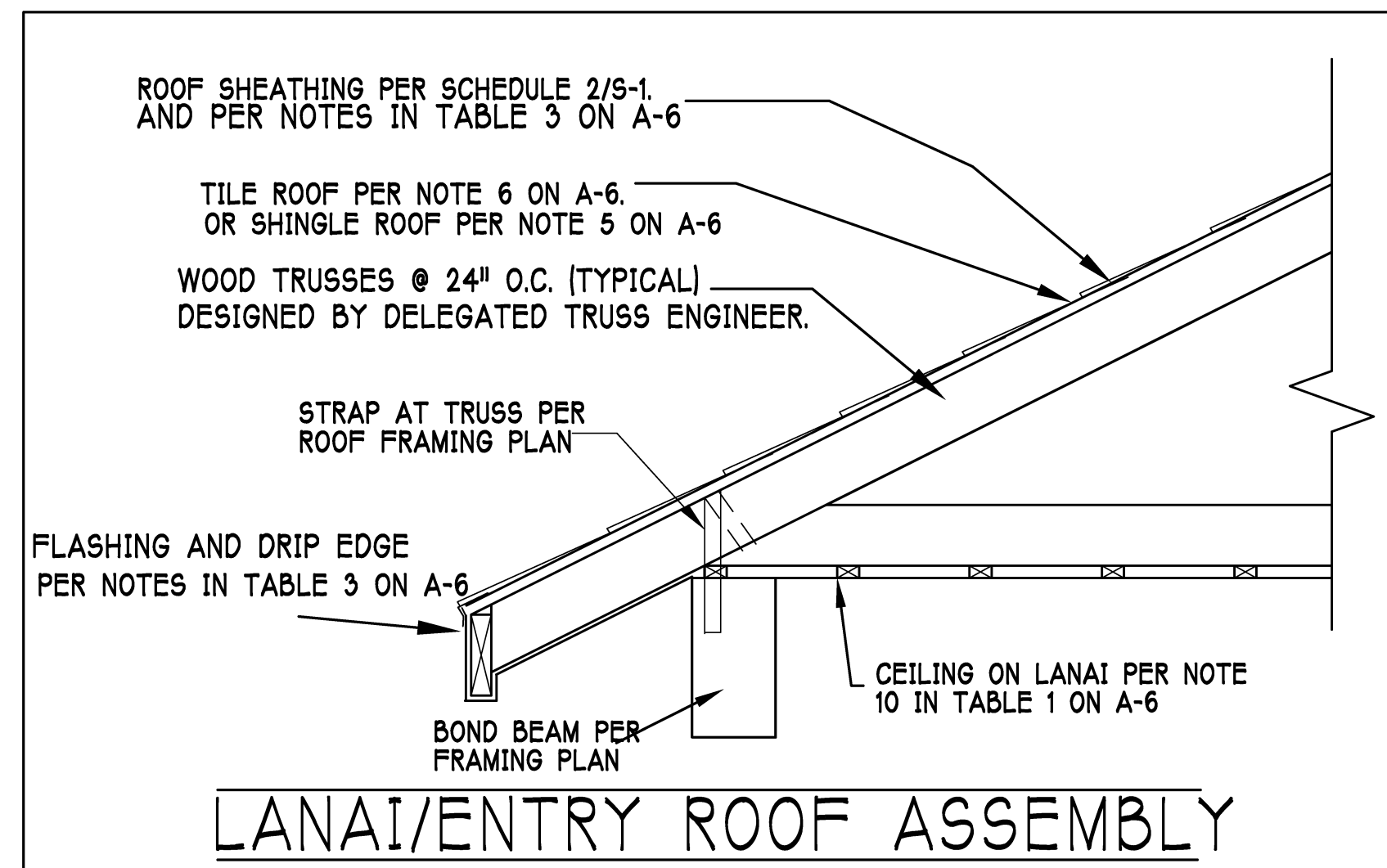
WINDOW JAM TO BLOCK DETAIL SCALE 1" = 1'-0"



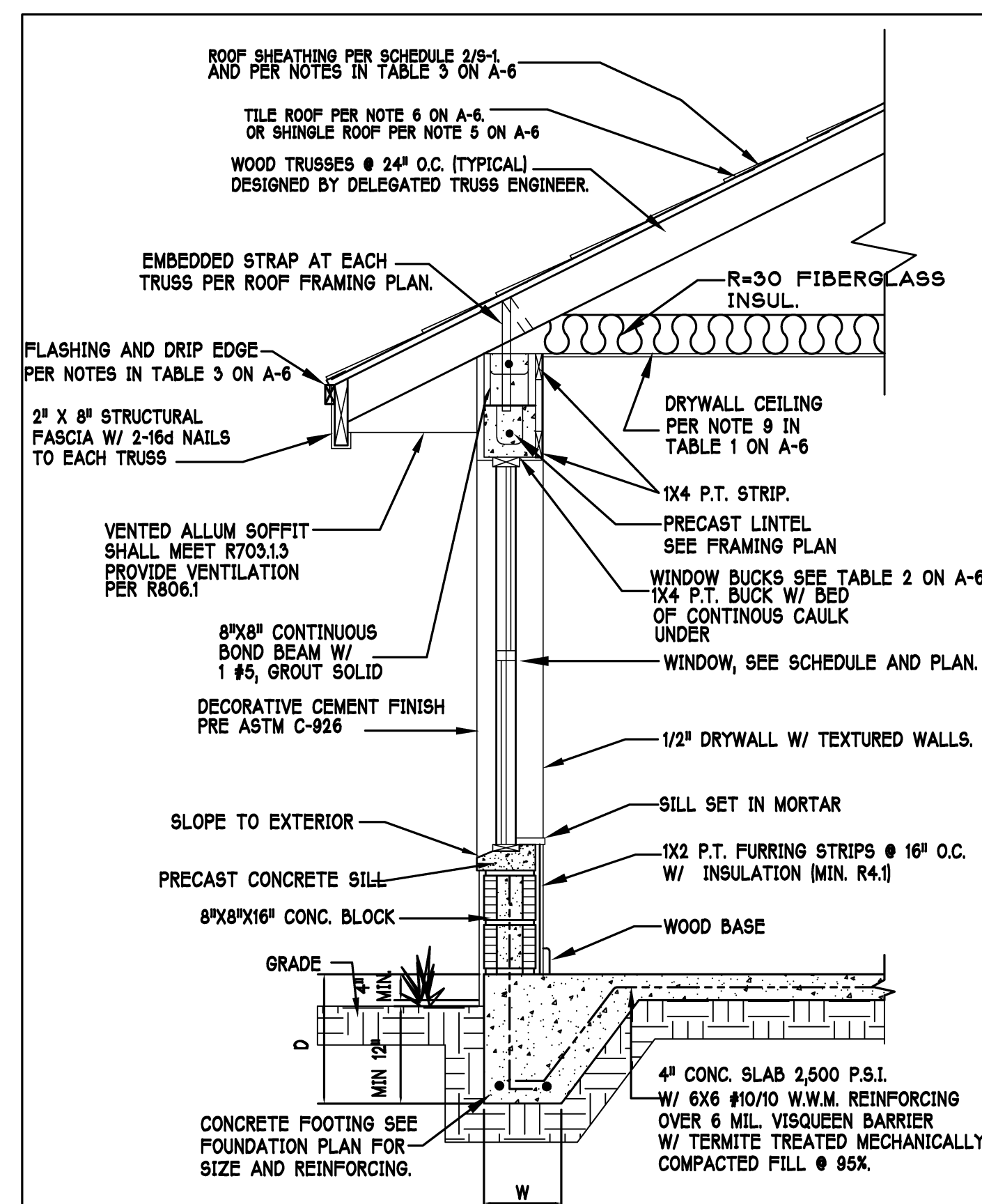
**GARAGE DOOR JAM DETAIL** SCALE 1 1/2" = 1'-0"



SL. GL. DR. JAM TO BLOCK DETAIL SCALE 1" = 1'-0"



WINDOW OR DOOR ARCH  
SPACE FRAMING ABOVE



## TYPICAL WALL SECTION

SCALE: 1/2"= 1'-0"



INSTALL AT ALL EXTERIOR WALL LOCATIONS WHERE  
WOOD STUD FRAMING IS ABOVE MASONRY WALLS.



PAN FLASHING PER R703.8.1.1  
SCALE: N.T.S.

R703.8.1.1 - Where flashing instructions or details are not provided by the window or door manufacturer or by the flashing manufacturer, "pan flashing" shall be installed at the sill of exterior window and door openings. Pan flashing shall be sealed or sloped in such a manner as to direct water to the surface of the exterior wall finish or to the water-resistive barrier for subsequent drainage. Openings using pan flashing shall also incorporate flashing or protection at the head and sides.

"Pan Flashing" is a generic term that used to refer to "metal pan flashing". However many modern materials have been developed for the same function such as:

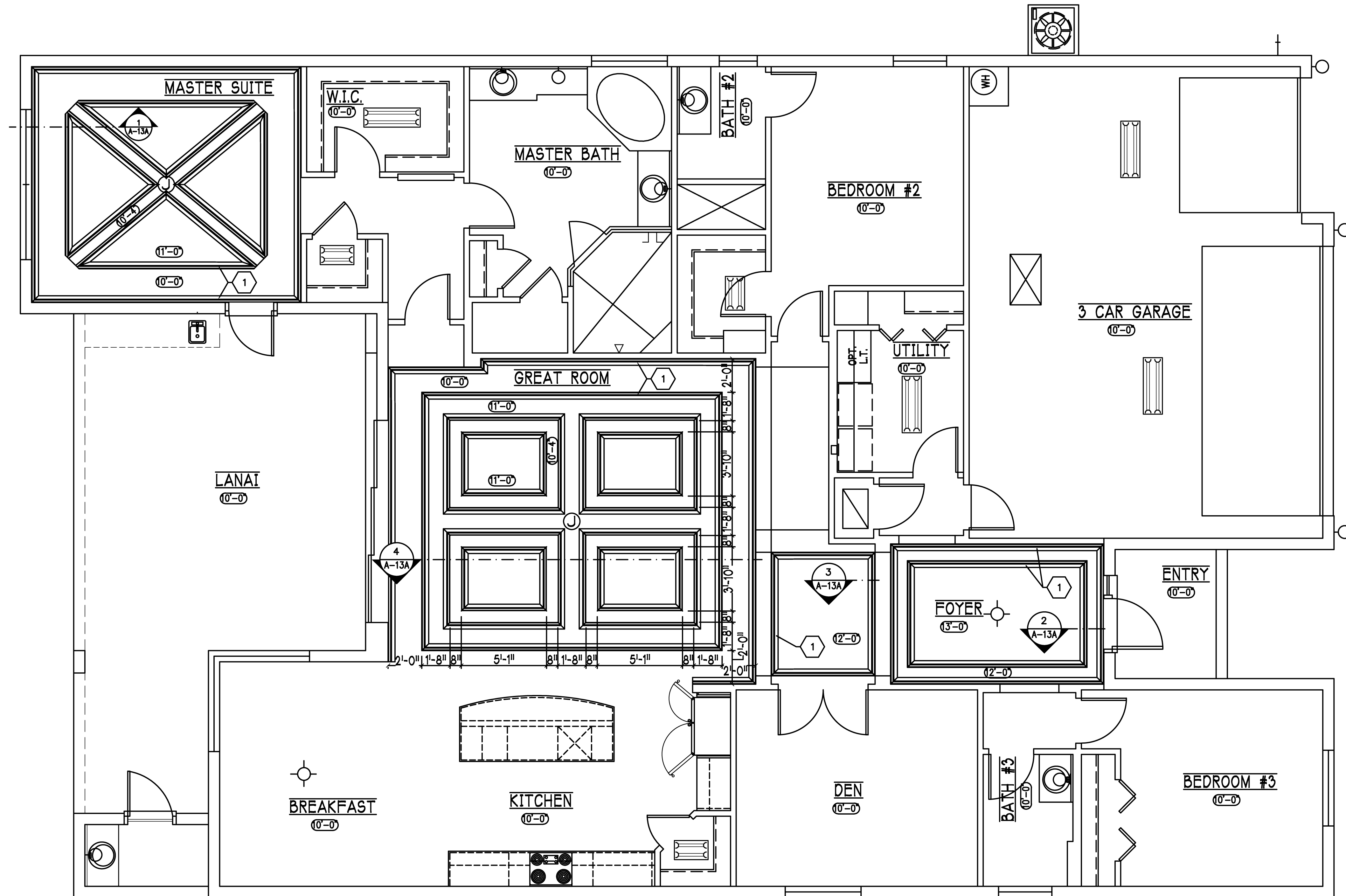
- Flexible Peel n Stick Flashing Membrane
- Fluid Applied Flashing

For such products, follow the manufacturer's installation instructions.

For in-depth flashing installation instructions, refer to the following publications:

G FMA/AAMA 100  
FMA/AAMA 200  
FMA/WDMA 250  
FMA/AAMA/WDMA 300

DESIGN IN ACCORDANCE W/ THE 2014 RESIDENTIAL  
FLORIDA BUILDING CODE- 5TH EDITION



DESIGN IN ACCORDANCE W/ THE 2014 RESIDENTIAL  
FLORIDA BUILDING CODE- 5TH EDITION

**D.R. HOUGHTON**   
*America's Builder*

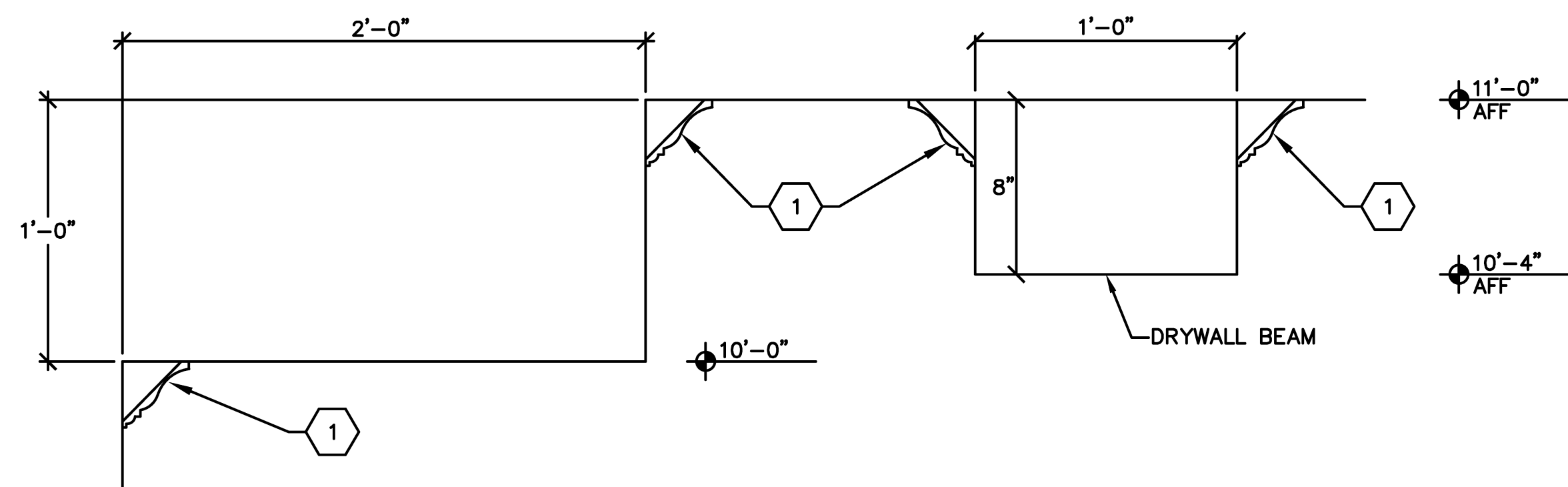
Gulf Coast Drafting  
& Design  
Phone (239) 540-1822  
Fax (239) 540-7759

LOT: BLOCK:  
SUBDIV:  
ADDRESS:  
G.C.D. JOB #: D.R.H.#:

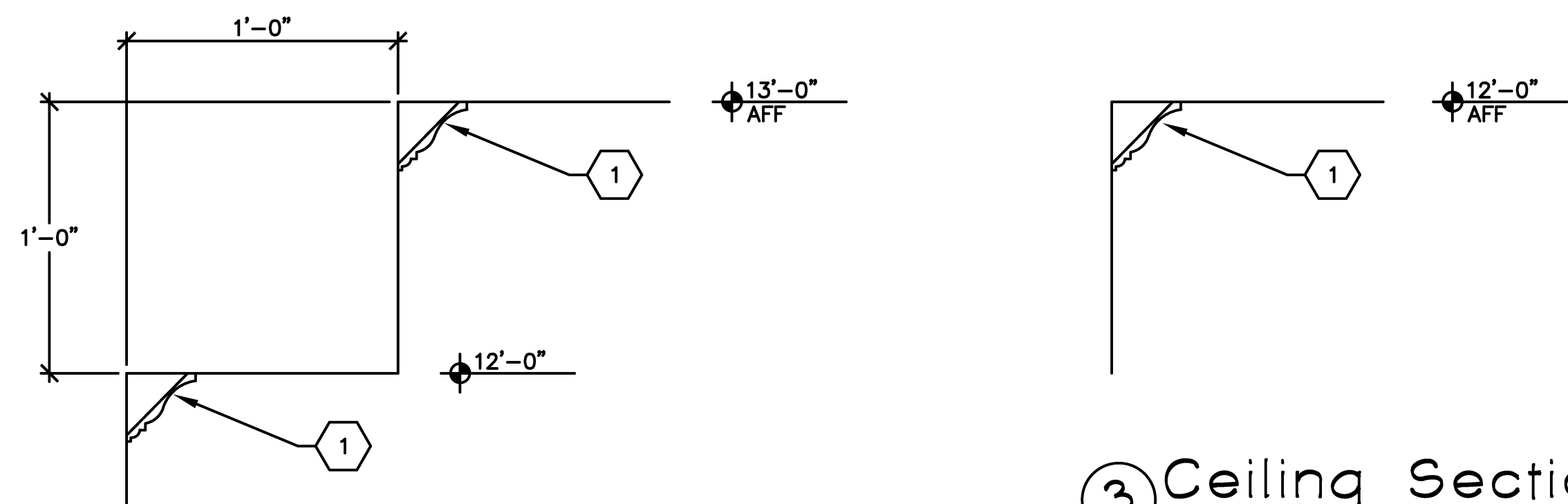
RESIDENCE FOR:  
**2583 EF OPTIONAL CEILING DETAILS**

DATE: 12-16-14  
DRAWN BY: GCH  
CHECKED BY: JWC  
REVISED:  
PLAN:  
SCALE: N.T.S.  
SHEET#

**A-8E-F**

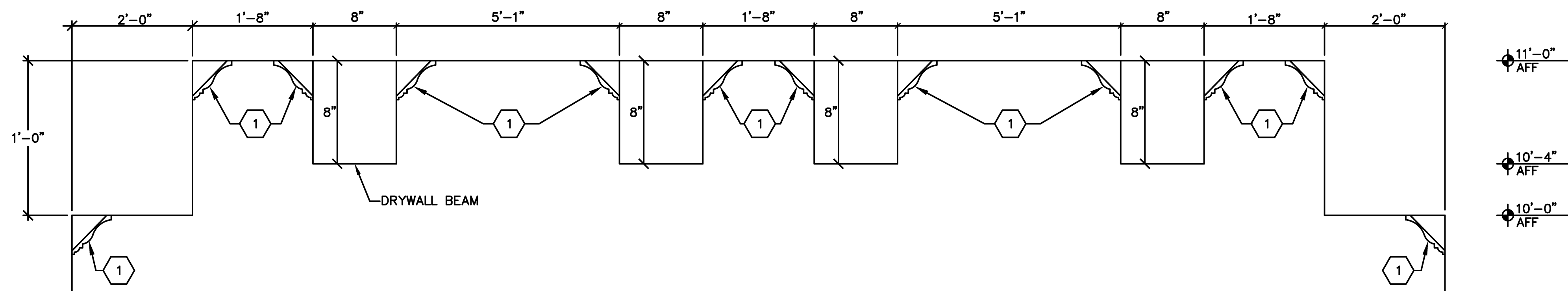


① Ceiling Section  
STANDARD CEILING

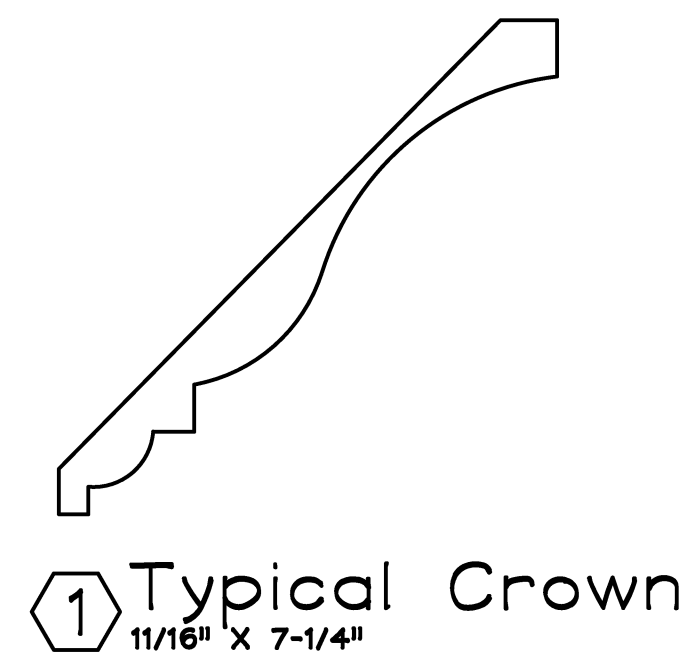


② Ceiling Section  
STANDARD CEILING

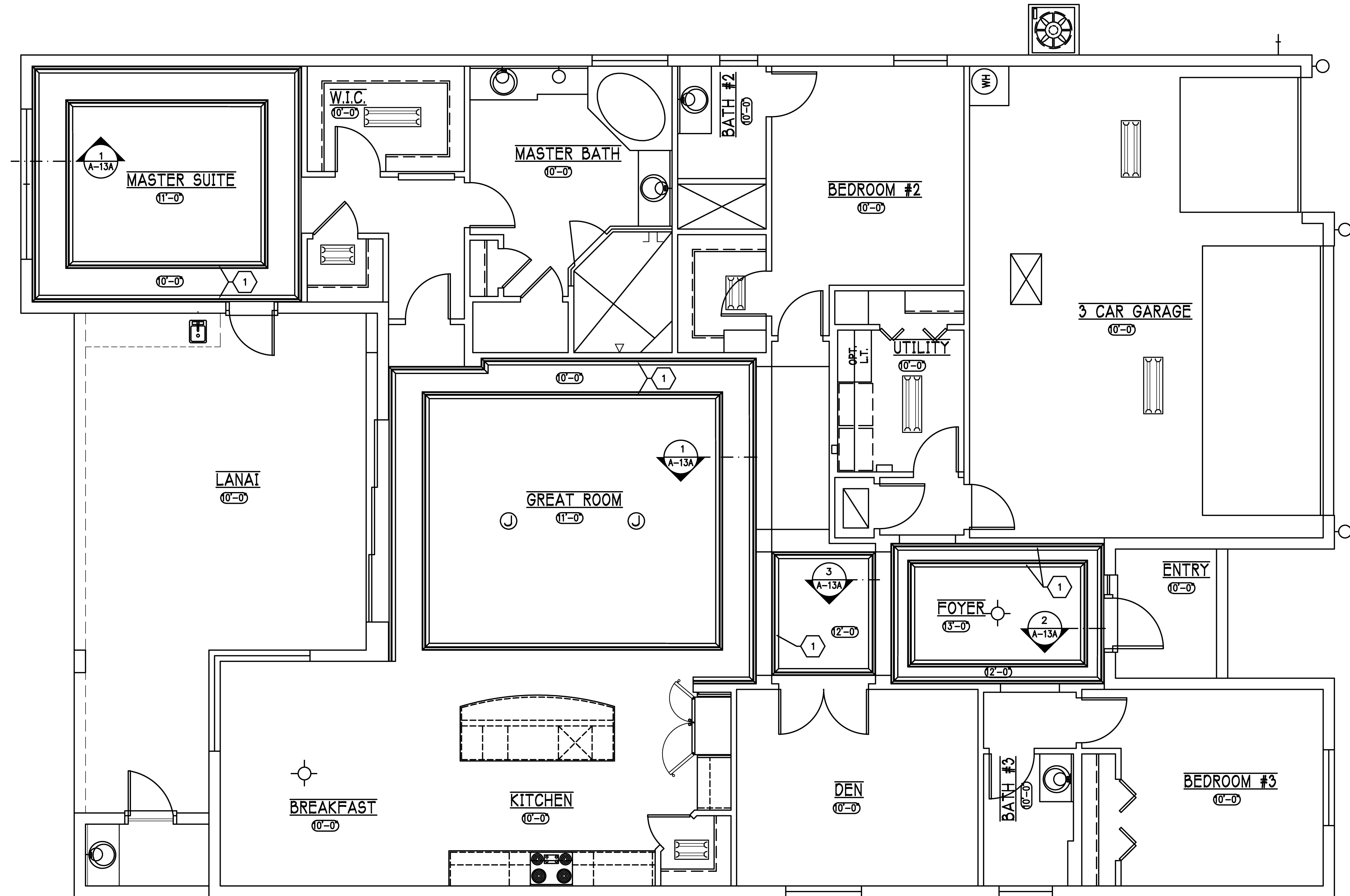
③ Ceiling Section  
STANDARD CEILING



④ Ceiling Section  
STANDARD CEILING

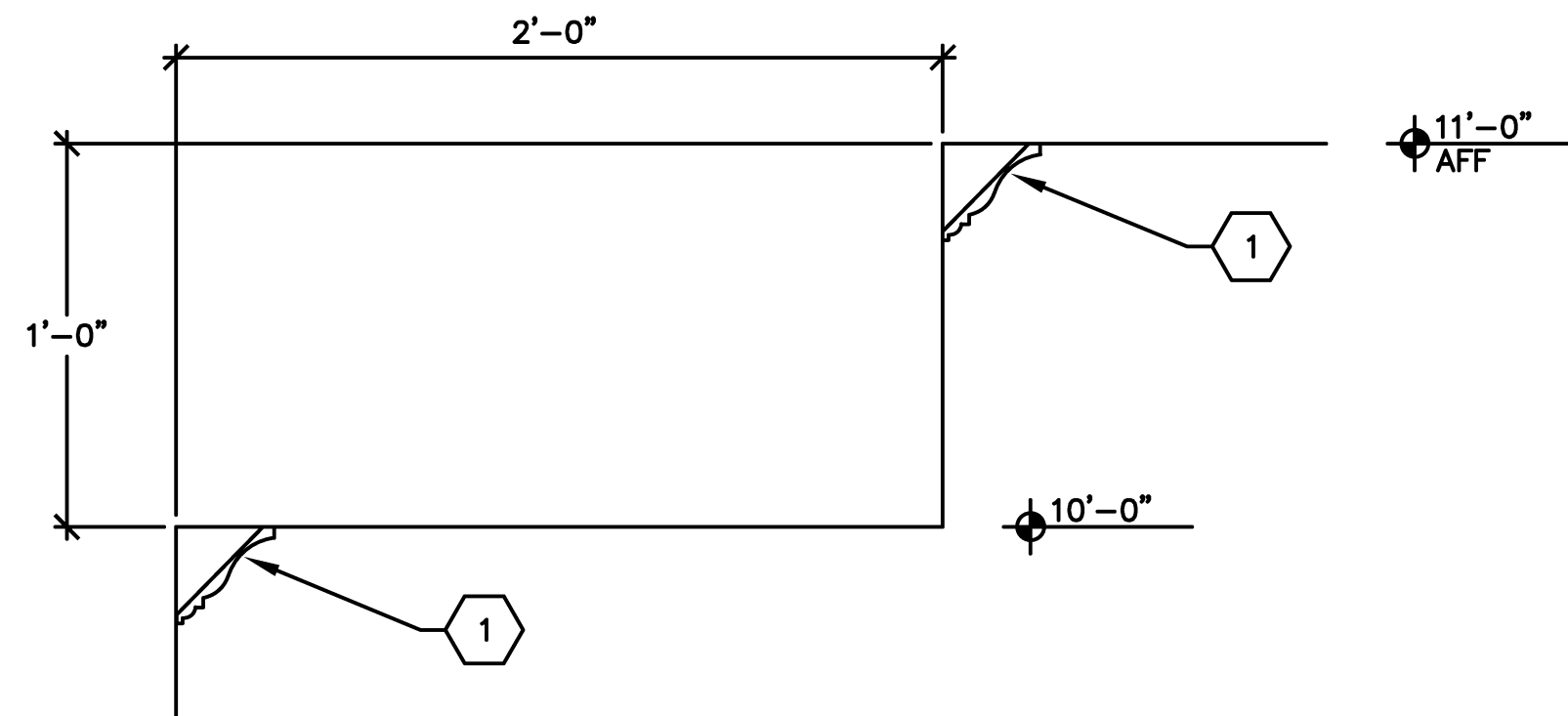


① Typical Crown  
11/16" X 7-1/4"

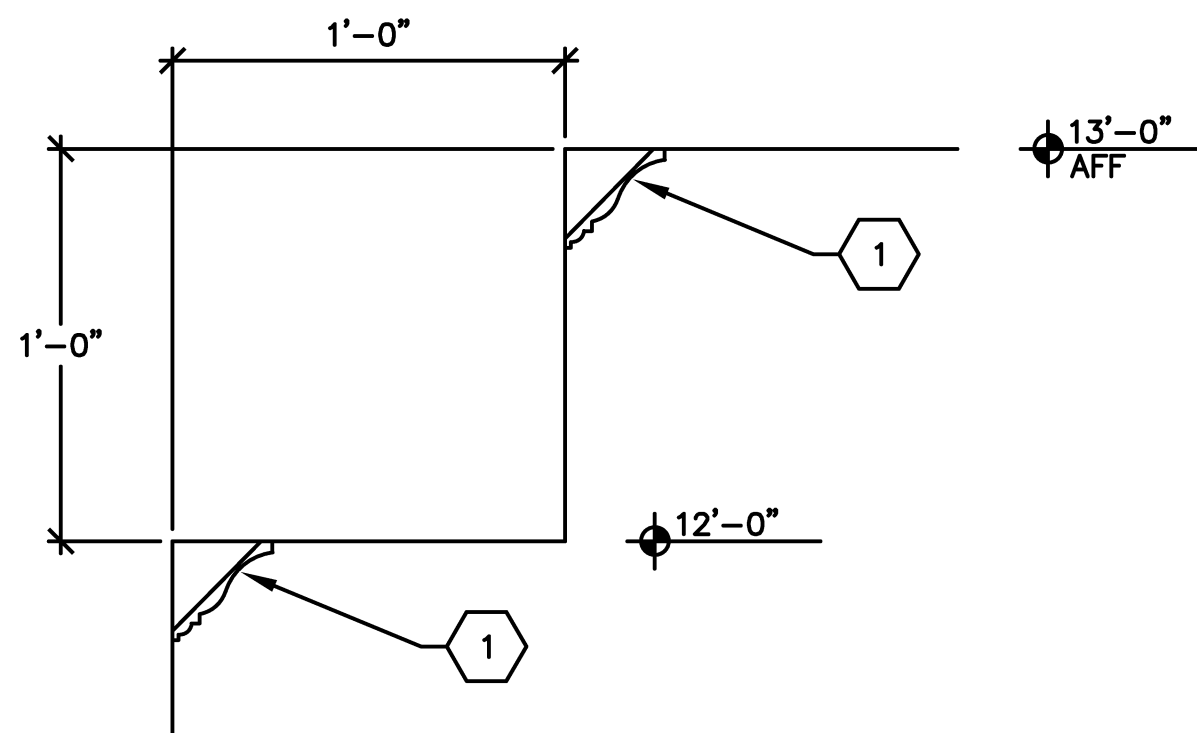


DESIGN IN ACCORDANCE W/ THE 2014 RESIDENTIAL  
FLORIDA BUILDING CODE- 5TH EDITION

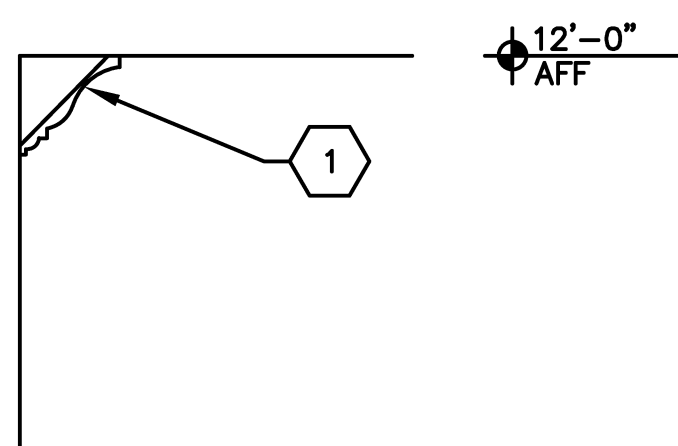
|   |                        |
|---|------------------------|
| <b>D. RHORTON</b><br><i>America's Builder</i>                                 |                        |
| Gulf Coast Drafting<br>& Design<br>Phone (239) 540-1822<br>Fax (239) 540-7759 |                        |
| RESIDENCE FOR:  | LOT: BLOCK:            |
| 2583 EF STANDARD CEILING DETAILS  | SUBDIV: ADDRESS:       |
|   | G.C.D. JOB #: D.R.H.#: |
|   |                        |
| DATE:   | 12-16-14               |
| DRAWN BY:   | GCH                    |
| CHECKED BY:   | JWC                    |
| REVISED:  |                        |
| PLAN:   |                        |
| SCALE:  | N.T.S.                 |
| SHEET#  | A-8E-F                 |



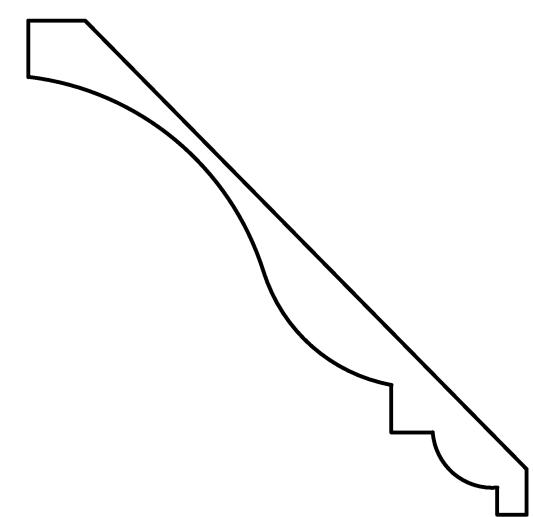
① Ceiling Section  
STANDARD CEILING



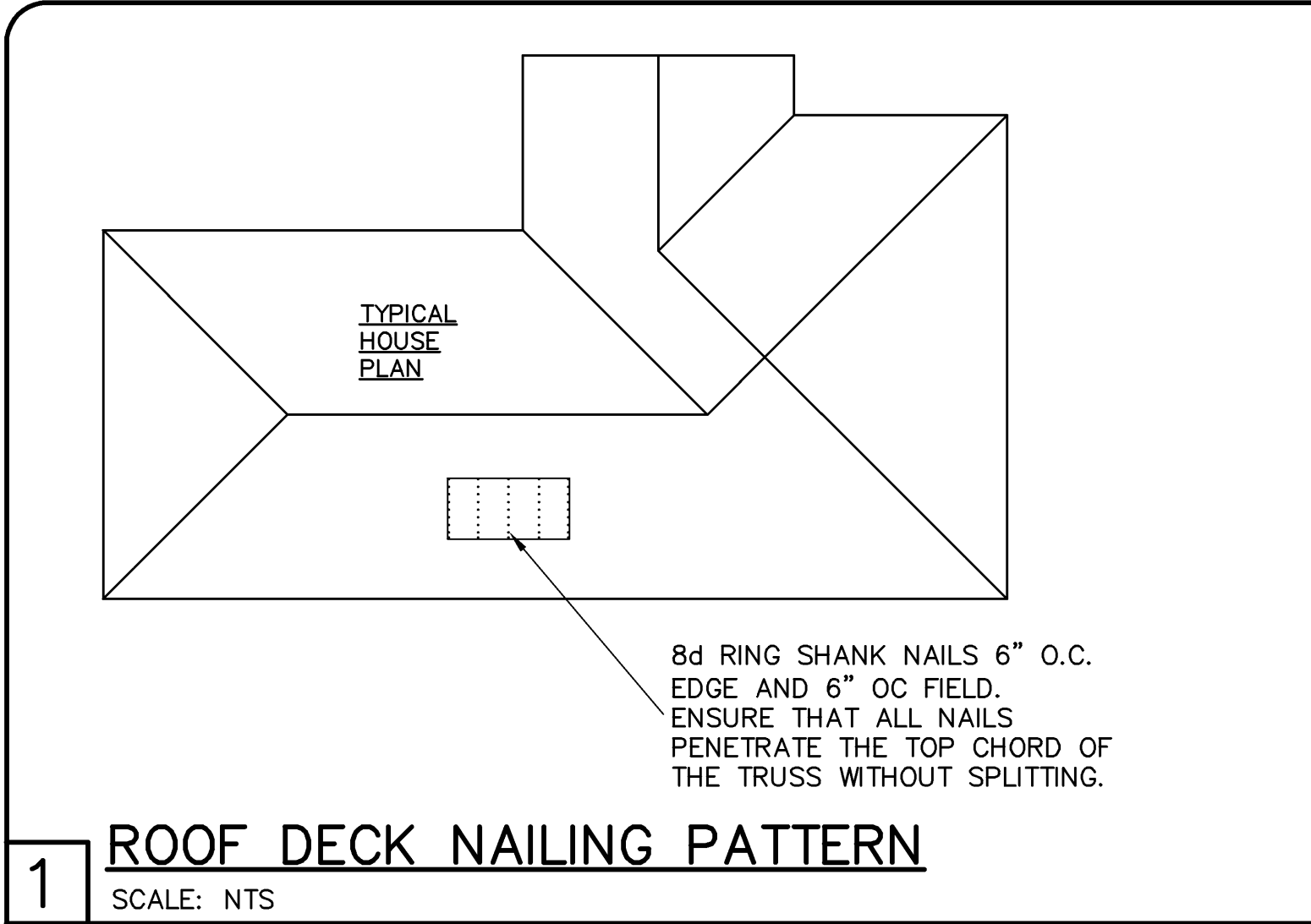
② Ceiling Section  
STANDARD CEILING



③ Ceiling Section  
STANDARD CEILING



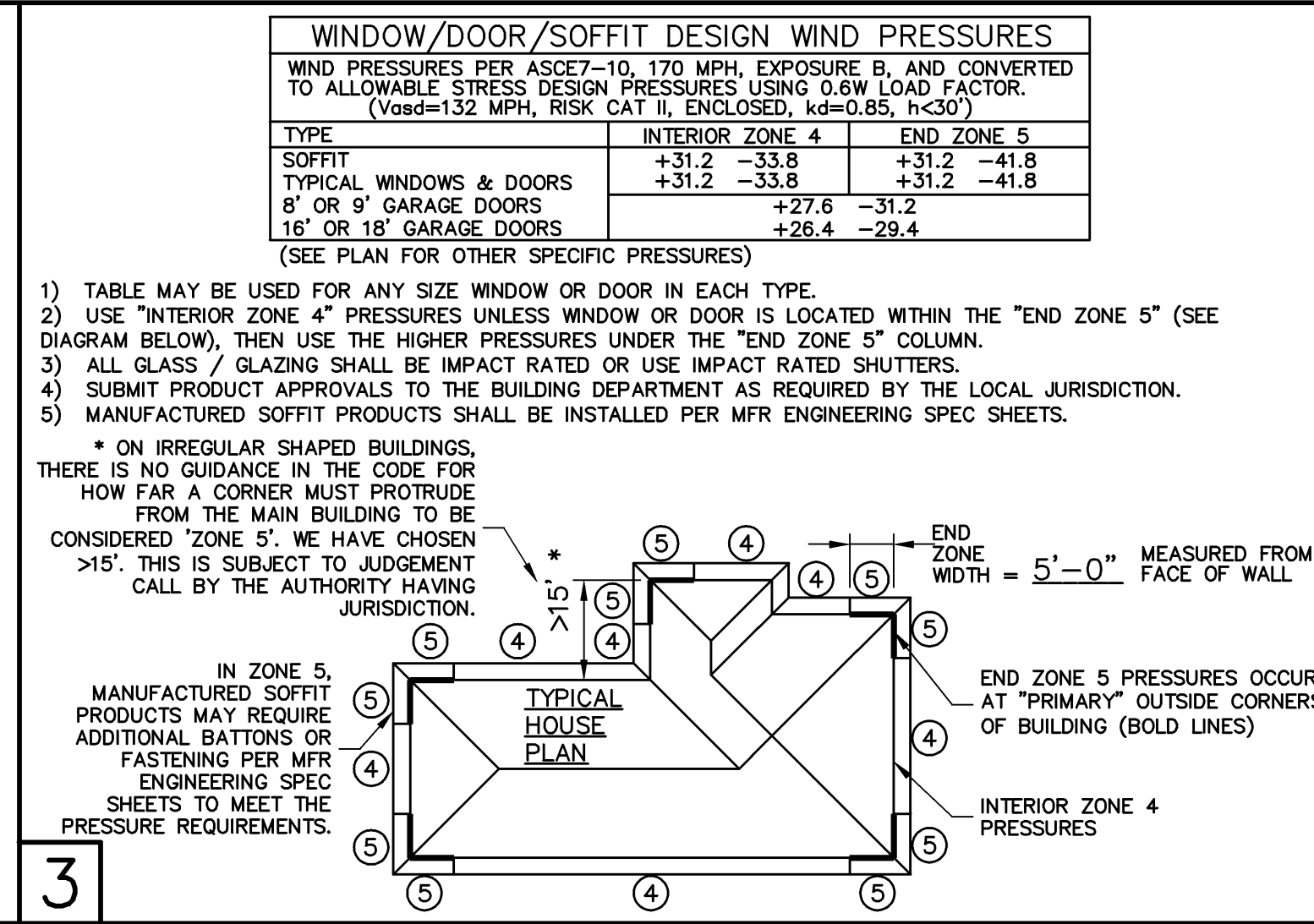
① Typical Crown  
11/16" X 7-1/4"



**2**

| SHEATHING SCHEDULE   |  |
|--|--|
| EXTERIOR STUD WALL   | FLOOR  |
| 7/16" ZIP SYSTEM WALL SHEATHING BY HUBER ENGINEERED WOODS LLC, NAILED W/ 8d COMMON WIRE @ 6" O.C. EDGE AND 6" O.C. FIELD. PROVIDE 2x4 BLOCKING AT ALL JOINTS. INSTALL SHEATHING AND SEAM TAPE IN STRICT ACCORDANCE WITH MFR. WRITTEN INSTRUCTIONS.   | N/A  |
| ROOF   | LANAI / ENTRY CEILING  |
| A.P.A. RATED SHEATHING, EXPOSURE 1, SPAN RATING 24/16 OR BETTER (HIGHER NUMBERS INDICATE BETTER SPAN RATING). THE USUAL CHOICE IS 15/32" CDX PLYWOOD OR 7/16" OSB, WITH THE REQUIRED APA GRADE MARKING. FASTEN WITH 8d RING SHANK NAILS @ 6" O.C. EDGE AND 6" O.C. FIELD.<br>(RING SHANK NAILS PER R803.2.3.1: 0.113" NOMINAL SHANK DIAMETER, RING DIA. OF 0.012" OVER SHANK DIAMETER, 16 TO 20 RINGS PER INCH, 0.280" DIAMETER FULL ROUND HEAD, 2" NAIL LENGTH) | OPTIONS:<br>1) 1x4 STRIPPING @ 16"OC w/ 2-8d NAILS TO EACH TRUSS, 5/8" EXTERIOR GYPBOARD CEILING, FASTEN w/8d NAILS OR 1 5/8" DRYWALL SCREWS @ 6"OC EDGE & FIELD.<br>2) 3/8" BC PLYWOOD NAILED w/ 6d COMMON @ 6" OC EDGE & FIELD.<br>3) WIRE LATHE AND 1/2" STUCCO. FASTEN WIRE LATHE WITH GALVANIZED STAPLES BY Senco OR EQUIV., 1" CROWN, 1" LONG, SPACED 4" OC. |

NOTE: EXTERIOR CEILINGS AND SOFFITS SPECIFIED HERE MEET THE DESIGN WIND PRESSURES PER R703.1.3.



DESIGN CRITERIA:

DESIGN IN ACCORDANCE WITH REQUIREMENTS OF THE FLORIDA BUILDING CODE 5th EDITION (2014) RESIDENTIAL

1. FLOOR & ROOF UNIFORM LOADS:  
ELEVATED FLOORS: LIVE LOAD 40 PSF, DEAD LOAD 20 PSF  
ROOF: LIVE TOP CHORD 20 PSF  
LIVE BOTTOM CHORD 10 PSF (NON-CONCURRENT w/ TOLL)  
CEMENT ROOF TILE DEAD LOAD 25 PSF TOTAL  
SHINGLE/METAL ROOFING DEAD LOAD 15 PSF TOTAL  
MINIMUM DEAD LOAD FOR WIND: TC 5 PSF, BC 5 PSF

DEFLECTION CRITERIA:  
FLOOR L/480 LIVE, L/360 TOTAL  
ROOF L/240 LIVE, L/180 TOTAL

2. WIND LOADS:  
WIND DESIGN PER, ASCE7-10  
BASIC WIND SPEED (ASCE7-10) 170 MPH  
NOMINAL WIND SPEED (Vg=132 MPH) 132 MPH  
BUILDING CATEGORY II  
IMPORTANCE FACTOR 1.00  
EXPOSURE B  
MEAN ROOF HEIGHT < 30 FT  
ROOF PITCH 6/12  
ENCLOSURE CLASS ENCLOSED  
INTERNAL PRESS. COEFF. +/- 0.18  
WINDOW/DOOR DESIGN WIND PRESSURE, SEE TABLE IN DETAIL 3.  
SOFFITS - PER R703.1.3, ALL SOFFITS SHALL BE CAPABLE OF RESISTING THE DESIGN PRESSURES SPECIFIED IN TABLE R301.2(2) FOR WALLS. PER R616.4, SOFFIT TESTING SHALL USE ASCE7 DESIGN PRESSURES USING 0.6W LOAD FACTOR.

3. REINFORCED CONCRETE:  
DESIGN AS PER ACI 318-11  
REQUIRED COMPRESSIVE STRENGTH AT 28 DAYS:  
SLAB ON GRADE f<sub>c</sub> = 2500 PSI  
3 1/2" MINIMUM THICKNESS REINFORCED WITH 6x6 w/1.4xw/1.4 WWF OR FIBERMESH.  
CONVENTIONAL SHALLOW FOOTINGS f<sub>c</sub> = 2500 PSI  
BEAMS AND COLUMNS f<sub>c</sub> = 3000 PSI  
ALL OTHER CONCRETE (U.N.O.) f<sub>c</sub> = 3000 PSI  
UNLESS OTHERWISE SHOWN ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:  
FOOTINGS 3"  
SLAB ON GRADE CENTERED  
BEAMS 1 1/2"  
COLUMNS 1 1/2"  
ALL REINFORCING STEEL SHALL BE PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAMS AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. ALL REINFORCING STEEL SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE.  
REINFORCING STEEL - ASTM A615 GRADE 40 FOR #3 GRADE 60 FOR #4 TO #11  
WELDED WIRE FABRIC - ASTM A185  
SPICES IN REINFORCING, SHALL BE 40 BAR DIAMETERS. NON-CONTACT LAP SPICES MAY BE USED PROVIDED REINFORCING IS NOT SPACED MORE THAN 5" APART FOR #5 BARS.  
FORMWORK AND SHORING SHALL REMAIN IN PLACE UNTIL CONCRETE HAS REACHED AT LEAST 2/3 OF THE REQUIRED 28 DAY STRENGTH.

4. REINFORCED MASONRY:  
DESIGN PER ACI 530-11  
REQUIRED COMPRESSIVE STRENGTHS:  
MASONRY WALLS f<sub>m</sub> = 1500 PSI

REINFORCING STEEL - ASTM A615 GRADE 60.  
SPICES IN REINFORCING, SHALL BE 48 BAR DIAMETERS.  
ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. ALL CELLS BELOW FINISHED GRADE SHALL BE GROUTED SOLID. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT AT DOT LOCATIONS ON PLAN. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN WALLS AT 16" OC VERTICALLY, UNLESS NOTED OTHERWISE. IN ADDITION, INSTALL JOINT REINFORCING IN THE FIRST TWO MORTAR JOINTS ABOVE AND BELOW OPENINGS, EXTENDING AT LEAST 24" BEYOND THE OPENING. LAP JOINT REINFORCING 6" MINIMUM.

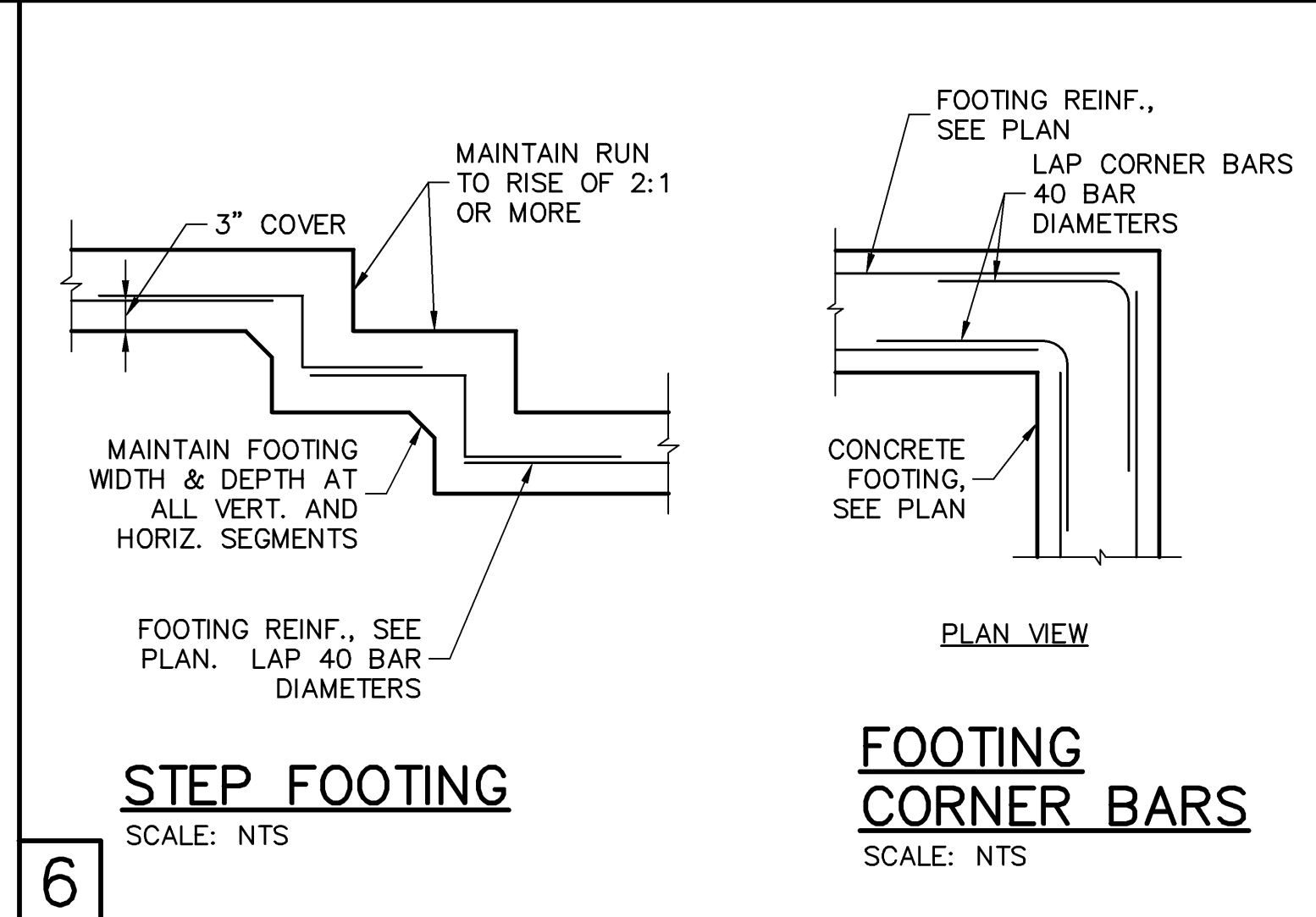
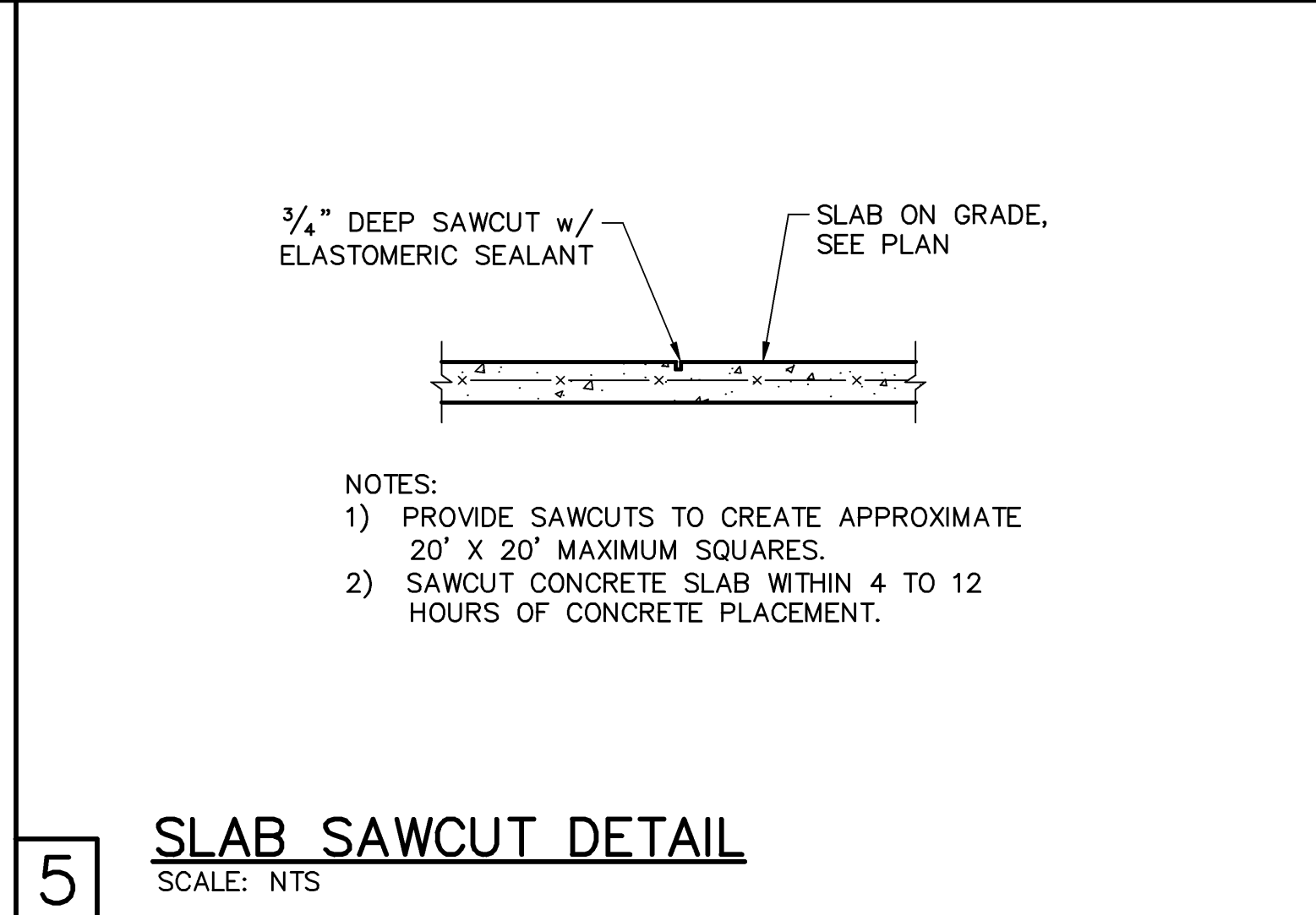
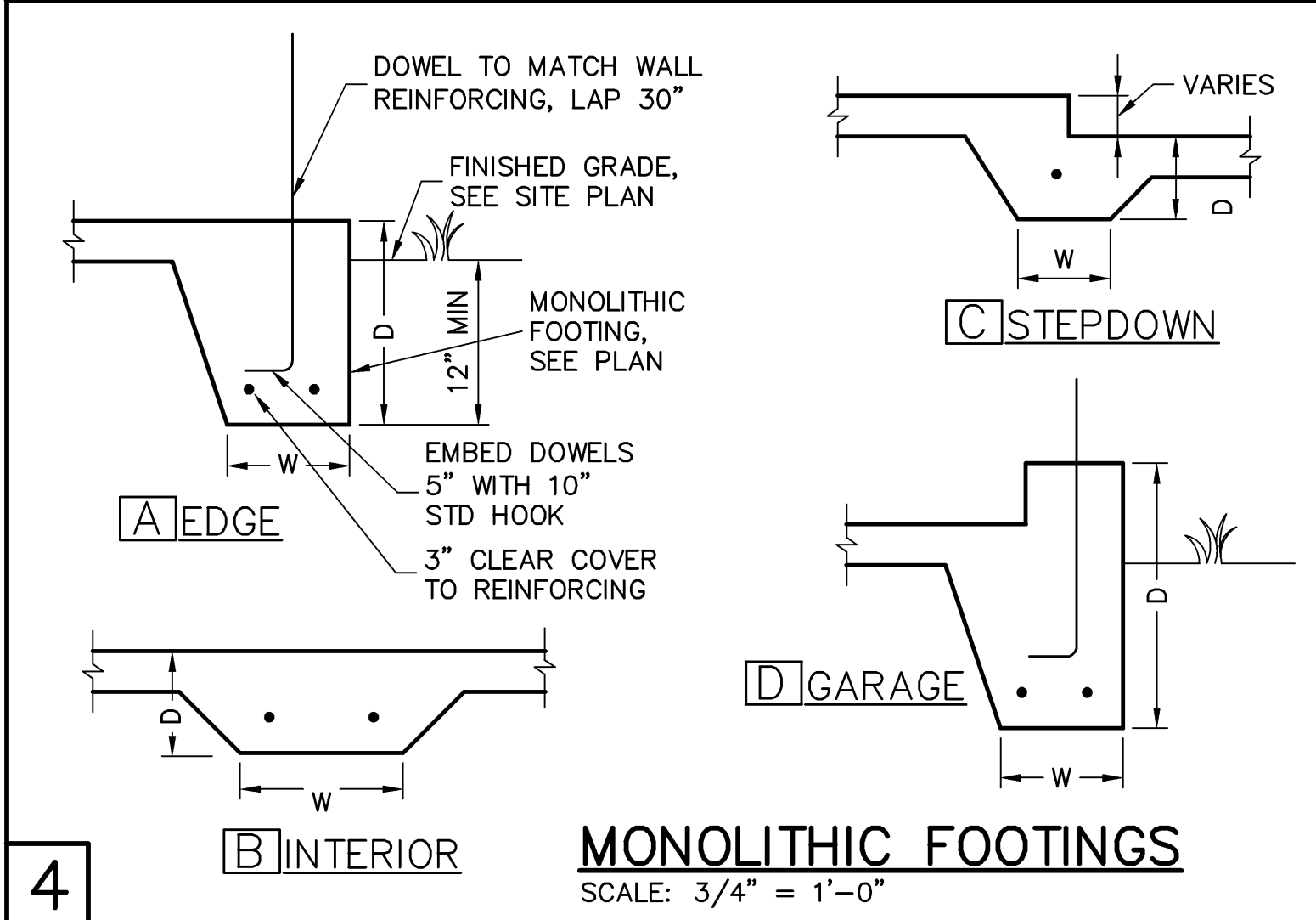
5. DELEGATED-ENGINEERED WOOD ROOF & FLOOR TRUSSES:  
ALL WOOD ROOF AND FLOOR TRUSSES SHALL BE DESIGNED BY A DELEGATED TRUSS ENGINEER PER RULE 61G15-31.003 OF THE FLORIDA ADMINISTRATIVE CODE. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91." FOR OTHER BRACING REQUIREMENTS, NOTIFY ENGINEER. PROVIDE PERMANENT BRACING PER TRUSS MFR. SHOP DRAWINGS. IF PERMANENT BRACING IS NOT SPECIFIED, CONTACT ENGINEER.

6. FOUNDATION:  
CONVENTIONAL SHALLOW CONCRETE FOOTINGS 2000 PSF SOIL BEARING CAPACITY.  
THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SOIL CONDITIONS FOR THE INTENDED STRUCTURE AND ASSUMED SOIL BEARING CAPACITY. IT IS RECOMMENDED THAT A GEOTECHNICAL FIRM BE HIRED TO PERFORM A SITE EVALUATION.

7. DIMENSIONS: VERIFY ALL DIMENSIONS WITH HOUSE PLANS. SEE HOUSE PLANS, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS.

8. MEANS AND METHODS: THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO SUPPORT STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

9. SHOP DRAWINGS: SHOP DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ENGINEER FOR REVIEW FOR ALL STRUCTURAL ELEMENTS UTILIZING PREFABRICATED COMPONENTS. ONE SET OF SIGNED & SEALED TRUSS ENGINEERING SHALL BE DELIVERED TO THE ENGINEER OF RECORD FOR THE STRUCTURE. PER FLORIDA ADMINISTRATIVE CODE 61G15-30.005 AND 61G15-31.003.



DESIGN CRITERIA:

DESIGN IN ACCORDANCE WITH REQUIREMENTS OF THE FLORIDA BUILDING CODE 5th EDITION (2014) RESIDENTIAL

1. FLOOR & ROOF UNIFORM LOADS:  
ELEVATED FLOORS: LIVE LOAD 40 PSF, DEAD LOAD 20 PSF  
ROOF: LIVE TOP CHORD 20 PSF  
LIVE BOTTOM CHORD 10 PSF (NON-CONCURRENT w/ TOLL)  
CEMENT ROOF TILE DEAD LOAD 25 PSF TOTAL  
SHINGLE/METAL ROOFING DEAD LOAD 15 PSF TOTAL  
MINIMUM DEAD LOAD FOR WIND: TC 5 PSF, BC 5 PSF

DEFLECTION CRITERIA:  
FLOOR L/480 LIVE, L/360 TOTAL  
ROOF L/240 LIVE, L/180 TOTAL

2. WIND LOADS:  
WIND DESIGN PER, ASCE7-10  
BASIC WIND SPEED (ASCE7-10) 170 MPH  
NOMINAL WIND SPEED (Vg=132 MPH) 132 MPH  
BUILDING CATEGORY II  
IMPORTANCE FACTOR 1.00  
EXPOSURE B  
MEAN ROOF HEIGHT < 30 FT  
ROOF PITCH 6/12  
ENCLOSURE CLASS ENCLOSED  
INTERNAL PRESS. COEFF. +/- 0.18  
WINDOW/DOOR DESIGN WIND PRESSURE, SEE TABLE IN DETAIL 3.  
SOFFITS - PER R703.1.3, ALL SOFFITS SHALL BE CAPABLE OF RESISTING THE DESIGN PRESSURES SPECIFIED IN TABLE R301.2(2) FOR WALLS. PER R616.4, SOFFIT TESTING SHALL USE ASCE7 DESIGN PRESSURES USING 0.6W LOAD FACTOR.

3. REINFORCED CONCRETE:  
DESIGN AS PER ACI 318-11  
REQUIRED COMPRESSIVE STRENGTH AT 28 DAYS:  
SLAB ON GRADE f<sub>c</sub> = 2500 PSI  
3 1/2" MINIMUM THICKNESS REINFORCED WITH 6x6 w/1.4xw/1.4 WWF OR FIBERMESH.  
CONVENTIONAL SHALLOW FOOTINGS f<sub>c</sub> = 2500 PSI  
BEAMS AND COLUMNS f<sub>c</sub> = 3000 PSI  
ALL OTHER CONCRETE (U.N.O.) f<sub>c</sub> = 3000 PSI  
UNLESS OTHERWISE SHOWN ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:  
FOOTINGS 3"  
SLAB ON GRADE CENTERED  
BEAMS 1 1/2"  
COLUMNS 1 1/2"  
ALL REINFORCING STEEL SHALL BE PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAMS AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. ALL REINFORCING STEEL SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE.  
REINFORCING STEEL - ASTM A615 GRADE 40 FOR #3 GRADE 60 FOR #4 TO #11  
WELDED WIRE FABRIC - ASTM A185  
SPICES IN REINFORCING, SHALL BE 40 BAR DIAMETERS. NON-CONTACT LAP SPICES MAY BE USED PROVIDED REINFORCING IS NOT SPACED MORE THAN 5" APART FOR #5 BARS.  
FORMWORK AND SHORING SHALL REMAIN IN PLACE UNTIL CONCRETE HAS REACHED AT LEAST 2/3 OF THE REQUIRED 28 DAY STRENGTH.

4. REINFORCED MASONRY:  
DESIGN PER ACI 530-11  
REQUIRED COMPRESSIVE STRENGTHS:  
MASONRY WALLS f<sub>m</sub> = 1500 PSI

REINFORCING STEEL - ASTM A615 GRADE 60.  
SPICES IN REINFORCING, SHALL BE 48 BAR DIAMETERS.  
ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. ALL CELLS BELOW FINISHED GRADE SHALL BE GROUTED SOLID. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT AT DOT LOCATIONS ON PLAN. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN WALLS AT 16" OC VERTICALLY, UNLESS NOTED OTHERWISE. IN ADDITION, INSTALL JOINT REINFORCING IN THE FIRST TWO MORTAR JOINTS ABOVE AND BELOW OPENINGS, EXTENDING AT LEAST 24" BEYOND THE OPENING. LAP JOINT REINFORCING 6" MINIMUM.

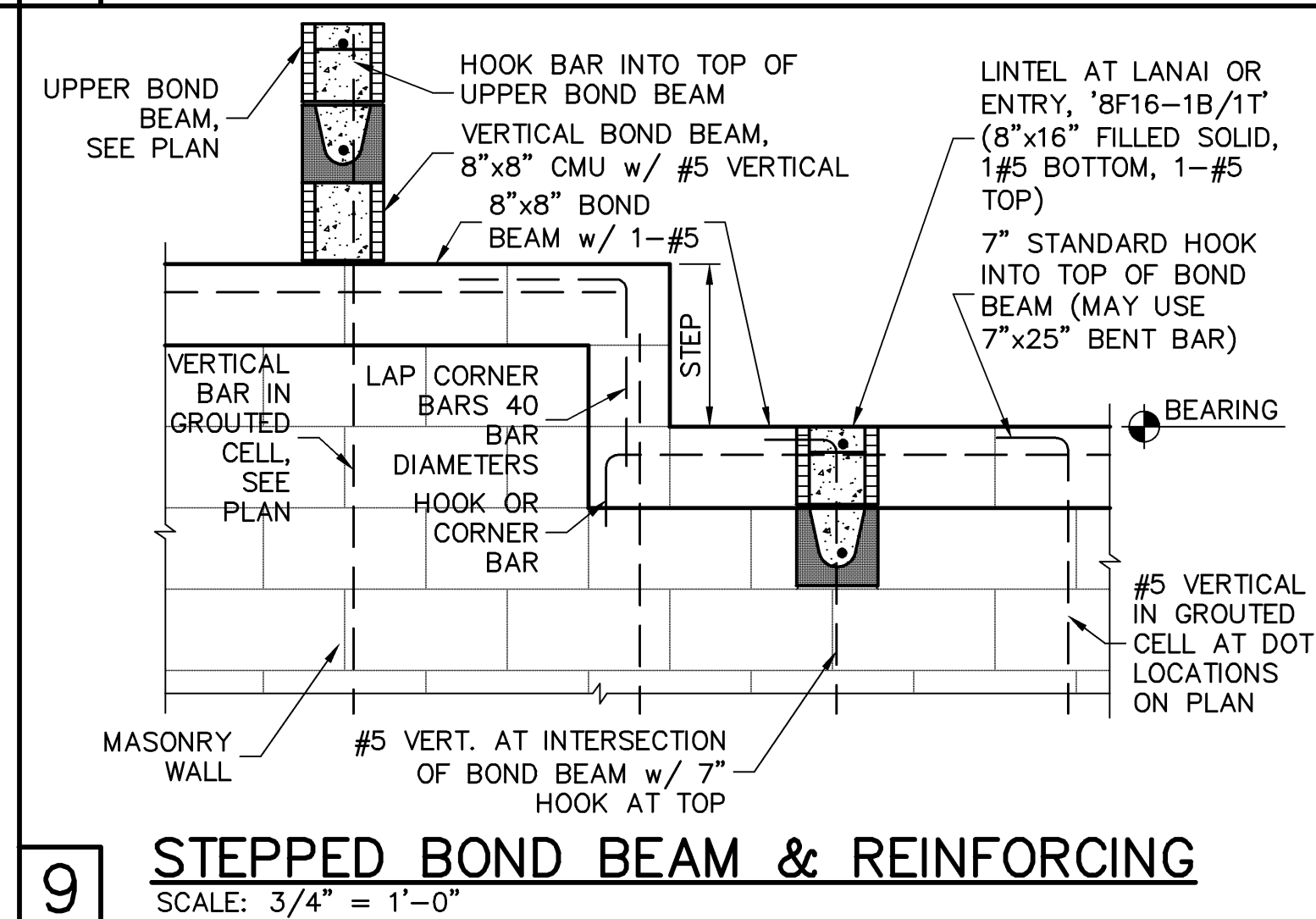
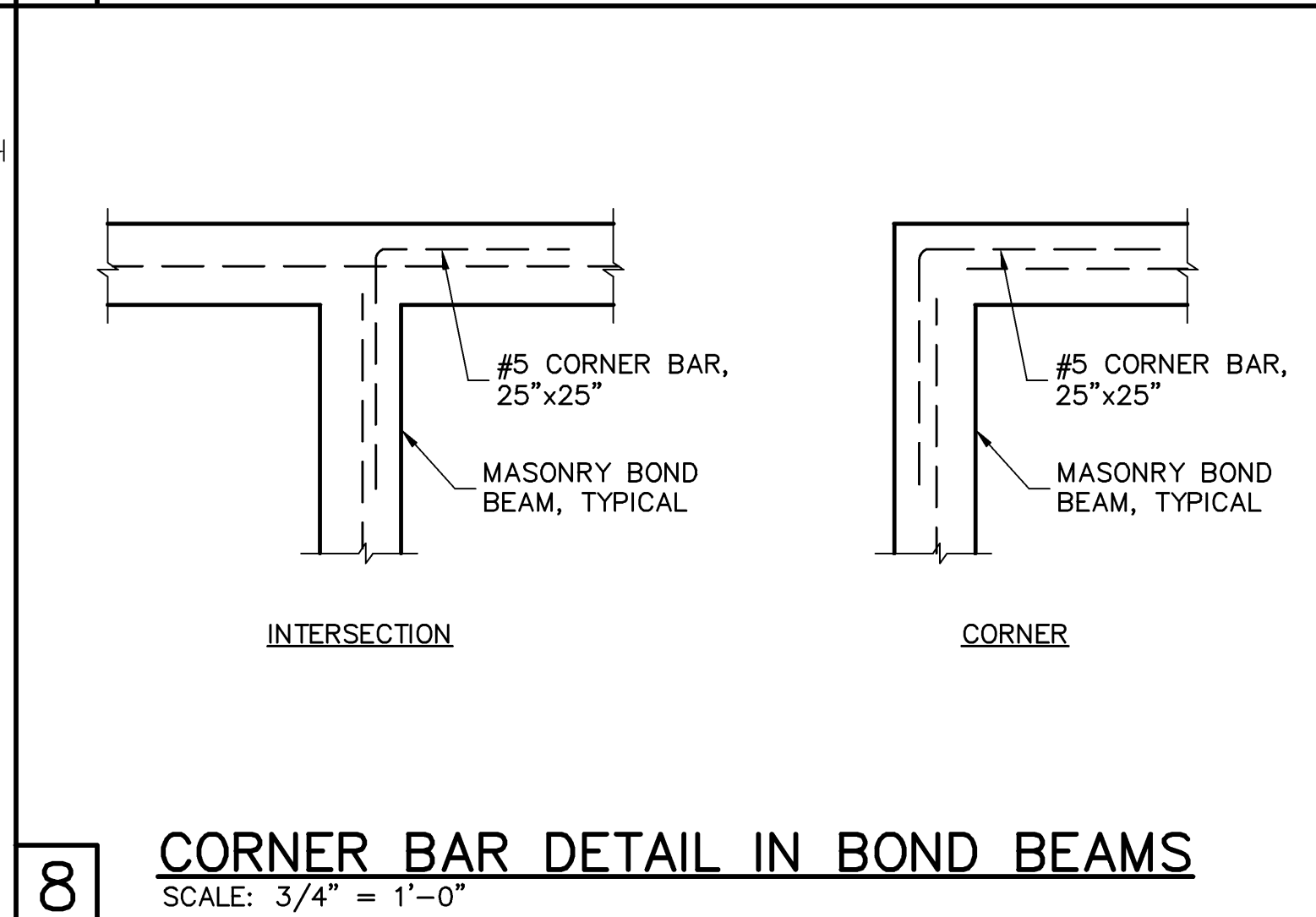
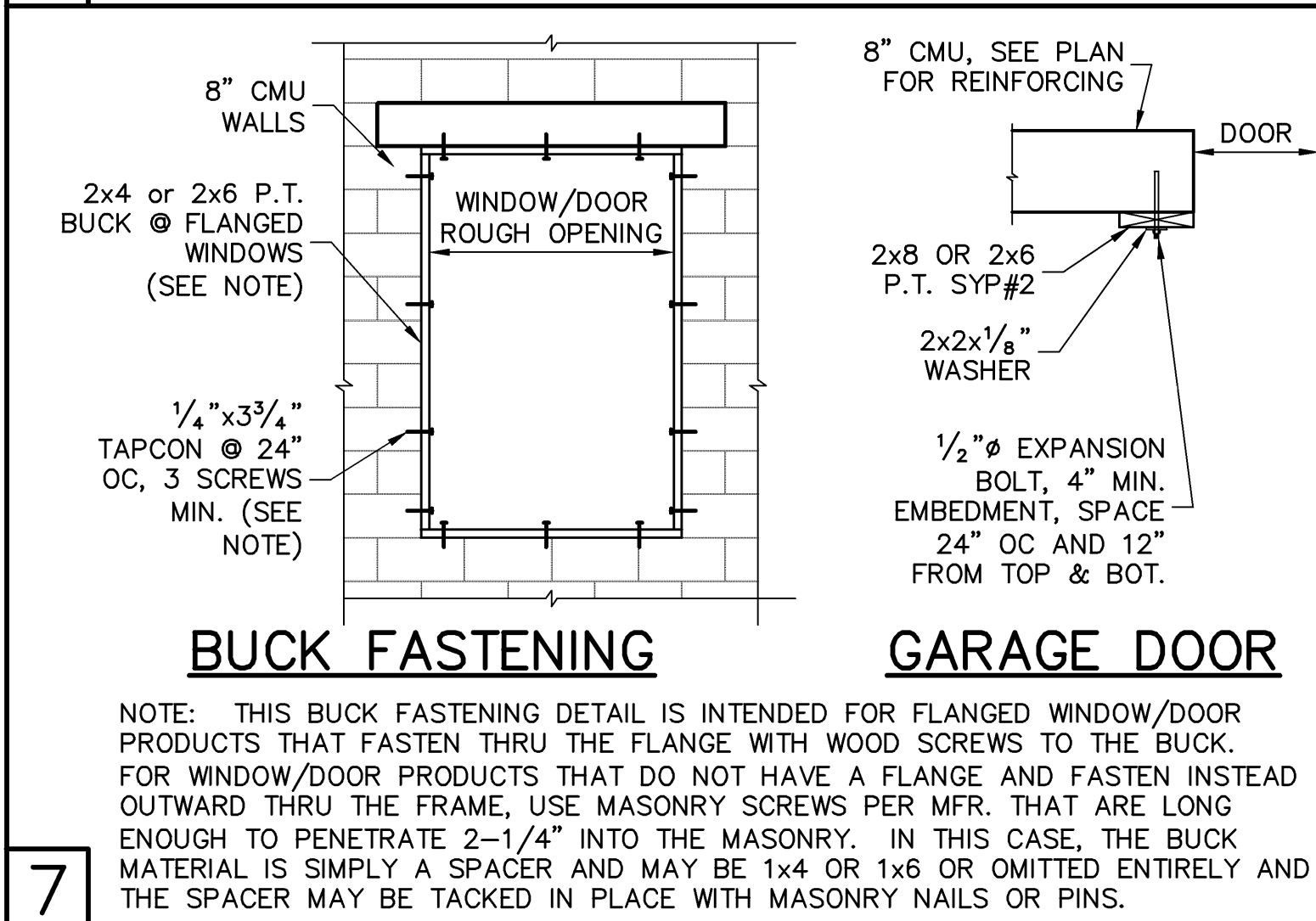
5. DELEGATED-ENGINEERED WOOD ROOF & FLOOR TRUSSES:  
ALL WOOD ROOF AND FLOOR TRUSSES SHALL BE DESIGNED BY A DELEGATED TRUSS ENGINEER PER RULE 61G15-31.003 OF THE FLORIDA ADMINISTRATIVE CODE. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91." FOR OTHER BRACING REQUIREMENTS, NOTIFY ENGINEER. PROVIDE PERMANENT BRACING PER TRUSS MFR. SHOP DRAWINGS. IF PERMANENT BRACING IS NOT SPECIFIED, CONTACT ENGINEER.

6. FOUNDATION:  
CONVENTIONAL SHALLOW CONCRETE FOOTINGS 2000 PSF SOIL BEARING CAPACITY.  
THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SOIL CONDITIONS FOR THE INTENDED STRUCTURE AND ASSUMED SOIL BEARING CAPACITY. IT IS RECOMMENDED THAT A GEOTECHNICAL FIRM BE HIRED TO PERFORM A SITE EVALUATION.

7. DIMENSIONS: VERIFY ALL DIMENSIONS WITH HOUSE PLANS. SEE HOUSE PLANS, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS.

8. MEANS AND METHODS: THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO SUPPORT STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR, OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE OF ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

9. SHOP DRAWINGS: SHOP DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ENGINEER FOR REVIEW FOR ALL STRUCTURAL ELEMENTS UTILIZING PREFABRICATED COMPONENTS. ONE SET OF SIGNED & SEALED TRUSS ENGINEERING SHALL BE DELIVERED TO THE ENGINEER OF RECORD FOR THE STRUCTURE. PER FLORIDA ADMINISTRATIVE CODE 61G15-30.005 AND 61G15-31.003.



DESIGN CRITERIA:

DESIGN IN ACCORDANCE WITH REQUIREMENTS OF THE FLORIDA BUILDING CODE 5th EDITION (2014) RESIDENTIAL

1. FLOOR & ROOF UNIFORM LOADS:  
ELEVATED FLOORS: LIVE LOAD 40 PSF, DEAD LOAD 20 PSF  
ROOF: LIVE TOP CHORD 20 PSF  
LIVE BOTTOM CHORD 10 PSF (NON-CONCURRENT w/ TOLL)  
CEMENT ROOF TILE DEAD LOAD 25 PSF TOTAL  
SHINGLE/METAL ROOFING DEAD LOAD 15 PSF TOTAL  
MINIMUM DEAD LOAD FOR WIND: TC 5 PSF, BC 5 PSF

DEFLECTION CRITERIA:  
FLOOR L/480 LIVE, L/360 TOTAL  
ROOF L/240 LIVE, L/180 TOTAL

2. WIND LOADS:  
WIND DESIGN PER, ASCE7-10  
BASIC WIND SPEED (ASCE7-10) 170 MPH  
NOMINAL WIND SPEED (Vg=132 MPH) 132 MPH  
BUILDING CATEGORY II  
IMPORTANCE FACTOR 1.00  
EXPOSURE B  
MEAN ROOF HEIGHT < 30 FT  
ROOF PITCH 6/12  
ENCLOSURE CLASS ENCLOSED  
INTERNAL PRESS. COEFF. +/- 0.18  
WINDOW/DOOR DESIGN WIND PRESSURE, SEE TABLE IN DETAIL 3.  
SOFFITS - PER R703.1.3, ALL SOFFITS SHALL BE CAPABLE OF RESISTING THE DESIGN PRESSURES SPECIFIED IN TABLE R301.2(2) FOR WALLS. PER R616.4, SOFFIT TESTING SHALL USE ASCE7 DESIGN PRESSURES USING 0.6W LOAD FACTOR.

3. REINFORCED CONCRETE:  
DESIGN AS PER ACI 318-11  
REQUIRED COMPRESSIVE STRENGTH AT 28 DAYS:  
SLAB ON GRADE f<sub>c</sub> = 2500 PSI  
3 1/2" MINIMUM THICKNESS REINFORCED WITH 6x6 w/1.4xw/1.4 WWF OR FIBERMESH.  
CONVENTIONAL SHALLOW FOOTINGS f<sub>c</sub> = 2500 PSI  
BEAMS AND COLUMNS f<sub>c</sub> = 3000 PSI  
ALL OTHER CONCRETE (U.N.O.) f<sub>c</sub> = 3000 PSI  
UNLESS OTHERWISE SHOWN ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:  
FOOTINGS 3"  
SLAB ON GRADE CENTERED  
BEAMS 1 1/2"  
COLUMNS 1 1/2"  
ALL REINFORCING STEEL SHALL BE PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAMS AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. ALL REINFORCING STEEL SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE.  
REINFORCING STEEL - ASTM A615 GRADE 40 FOR #3 GRADE 60 FOR #4 TO #11  
WELDED WIRE FABRIC - ASTM A185  
SPICES IN REINFORCING, SHALL BE 40 BAR DIAMETERS. NON-CONTACT LAP SPICES MAY BE USED PROVIDED REINFORCING IS NOT SPACED MORE THAN 5" APART FOR #5 BARS.  
FORMWORK AND SHORING SHALL REMAIN IN PLACE UNTIL CONCRETE HAS REACHED AT LEAST 2/3 OF THE REQUIRED 28 DAY STRENGTH.

4. REINFORCED MASONRY:  
DESIGN PER ACI 530-11  
REQUIRED COMPRESSIVE STRENGTHS:  
MASONRY WALLS f<sub>m</sub> = 1500 PSI

REINFORCING STEEL - ASTM A615 GRADE 60.  
SPICES IN REINFORCING, SHALL BE 48 BAR DIAMETERS.  
ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. ALL CELLS BELOW FINISHED GRADE SHALL BE GROUTED SOLID. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT AT DOT LOCATIONS ON PLAN. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN WALLS AT 16" OC VERTICALLY, UNLESS NOTED OTHERWISE. IN ADDITION, INSTALL JOINT REINFORCING IN THE FIRST TWO MORTAR JOINTS ABOVE AND BELOW OPENINGS, EXTENDING AT LEAST 24" BEYOND THE OPENING. LAP JOINT REINFORCING 6" MINIMUM.

5. DELEGATED-ENGINEERED WOOD ROOF & FLOOR TRUSSES:  
ALL WOOD ROOF AND FLOOR TRUSSES SHALL BE DESIGNED BY A DELEGATED TRUSS ENGINEER PER RULE 61G15-31.003 OF THE FLORIDA ADMINISTRATIVE CODE. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91." FOR OTHER BRACING REQUIREMENTS, NOTIFY ENGINEER. PROVIDE PERMANENT BRACING PER TRUSS MFR. SHOP DRAWINGS. IF PERMANENT BRACING IS NOT SPECIFIED, CONTACT ENGINEER.

6. FOUNDATION:  
CONVENTIONAL SHALLOW CONCRETE FOOTINGS 2000 PSF SOIL BEARING CAPACITY.  
THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THE SOIL CONDITIONS FOR THE INTENDED STRUCTURE AND ASSUMED SOIL BEARING CAPACITY. IT IS RECOMMENDED THAT A GEOTECHNICAL FIRM BE HIRED TO PERFORM A SITE EVALUATION.

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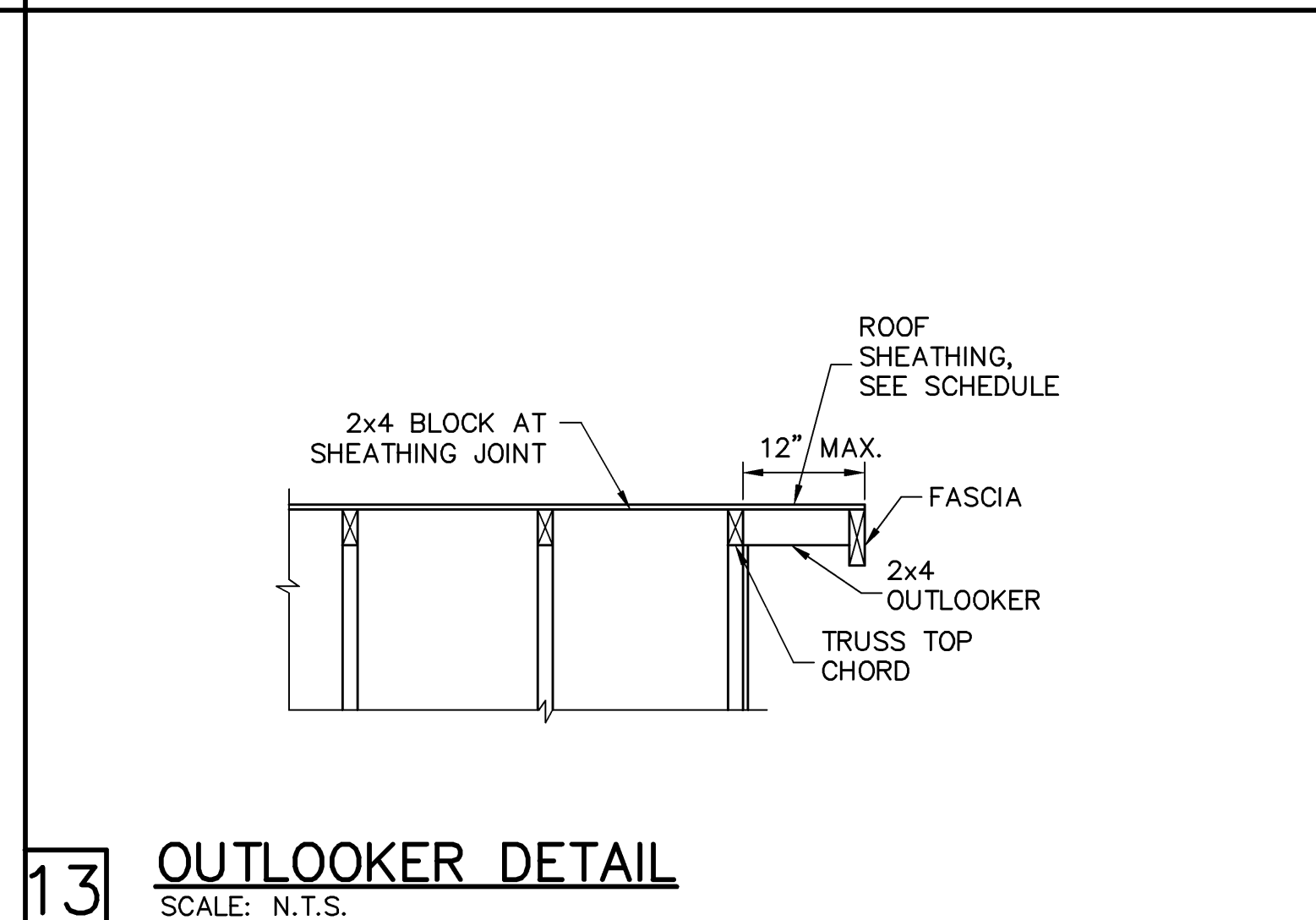
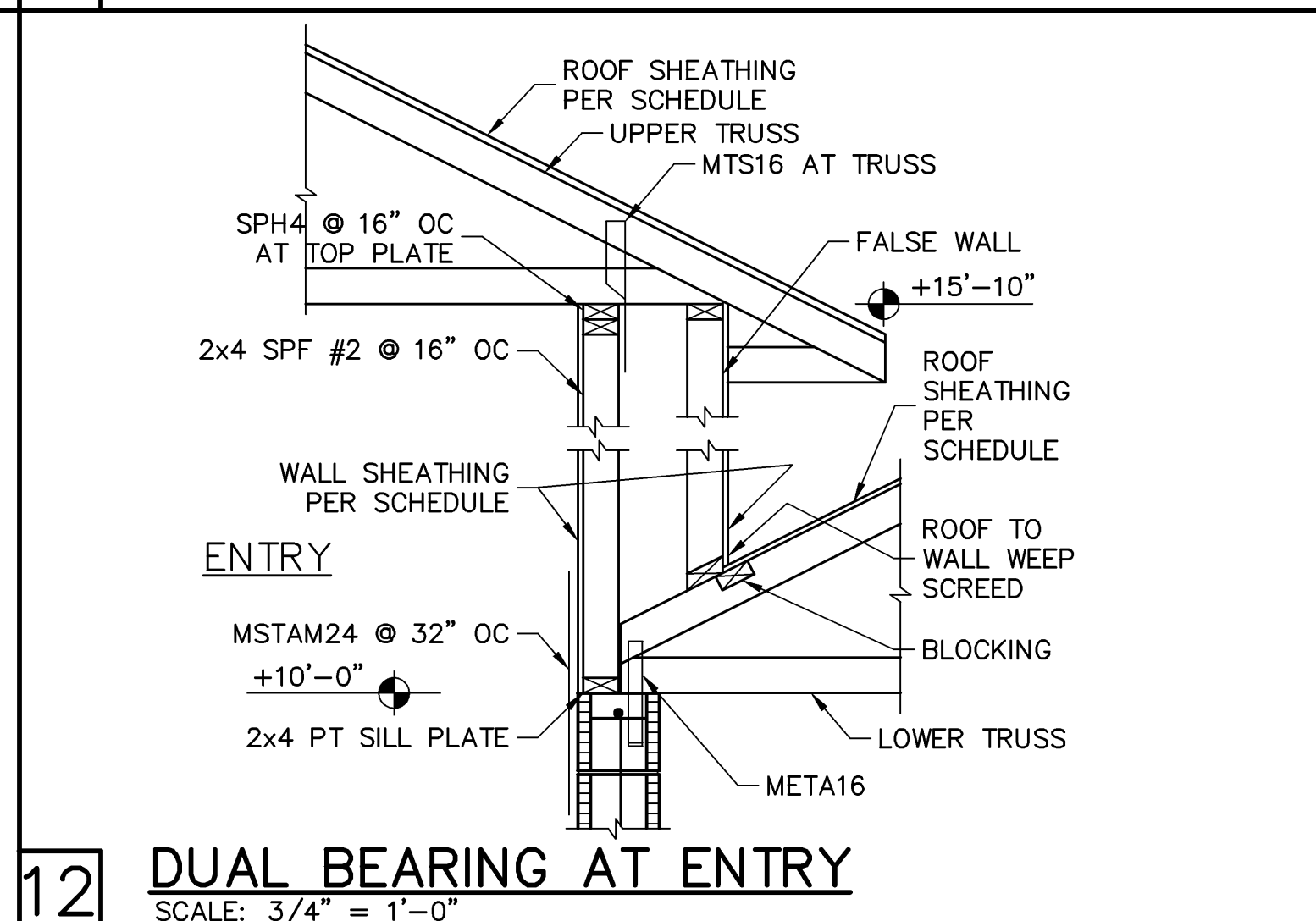
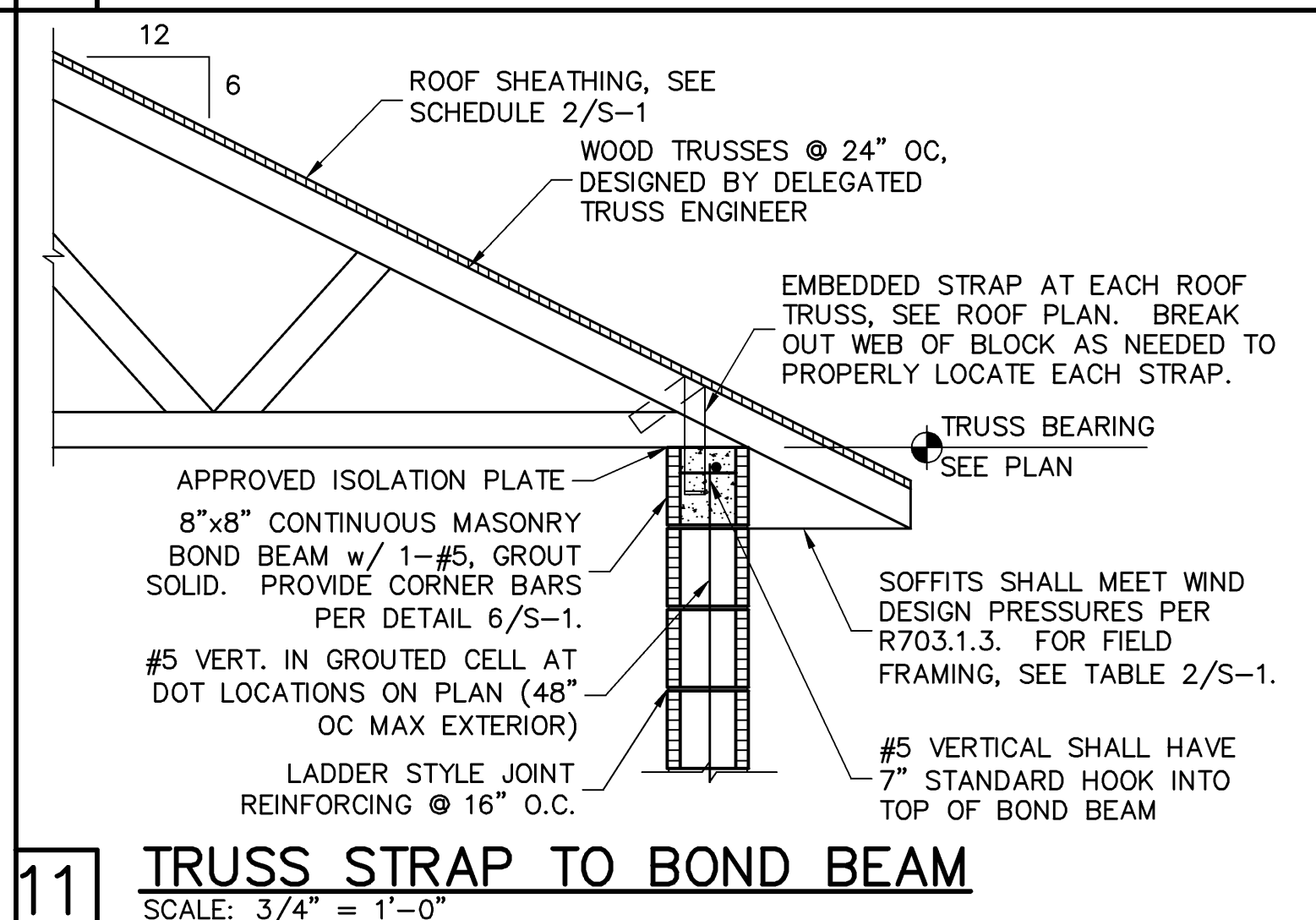
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**10**

**RETROFIT UPLIFT CONNECTOR SCHEDULE**

| TRUSS UPLIFT (LBS) @ 24" OC | CONNECTOR      |
|-----------------------------|----------------|
| TO 840                      | 1-MTSM16 or 20 |
| TO 1045                     | 1-HTSM16 or 20 |
| TO 2090                     | 2-HTSM16 or 20 |
| TO 4300                     | 2-LGT2         |
| TO 3480                     | HTT16          |
| TO 10530                    | HGT-2/3        |

NOTES:  
1) WHERE EMBEDDED STRAP IS MISSING OR MIS-LOCATED, PROVIDE A STRAP FROM THE ABOVE LIST AT EACH ROOF TRUSS BEARING POINT, BASED ON THE TRUSS UPLIFT VALUES IN THE SIGNED AND SEALED TRUSS DESIGN PACKAGE.  
2) CONNECTORS ARE SIMPSON STRONG TIE. ALL CONNECTORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH SIMPSON PRINTED INSTRUCTIONS.



REVISIONS

BY

STRUCTURAL ENGINEERING:

**STRUCTURAL SYSTEMS OF NORTH FLORIDA**

1634 S.E. 47th STREET, SUITE #3  
CAPE CORAL, FL 33904  
(239) 549-4554  
CA# 8829

DESIGNED IN ACCORDANCE WITH FLORIDA BUILDING CODE 5th EDITION (2014) RESIDENTIAL

BUILDER:

**D.R. HOHON**

*America's Builder*

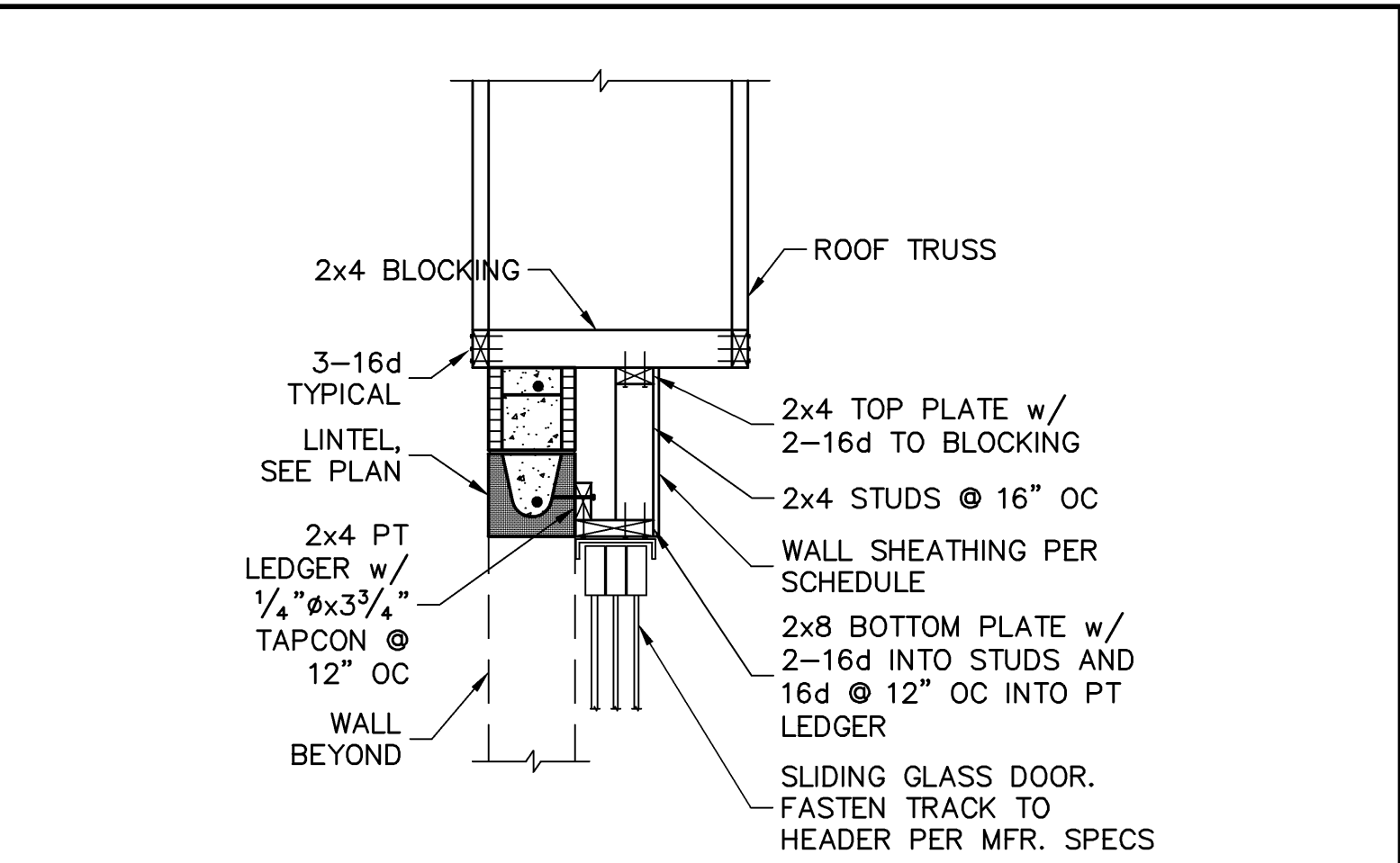
STRUCTURAL DETAILS FOR  
MODEL 2583 E  
EXTENDED LANAI

9479 LACOMAR COURT  
NAPLES, FLORIDA  
SUBDIVISION: FIDDLER'S CREEK

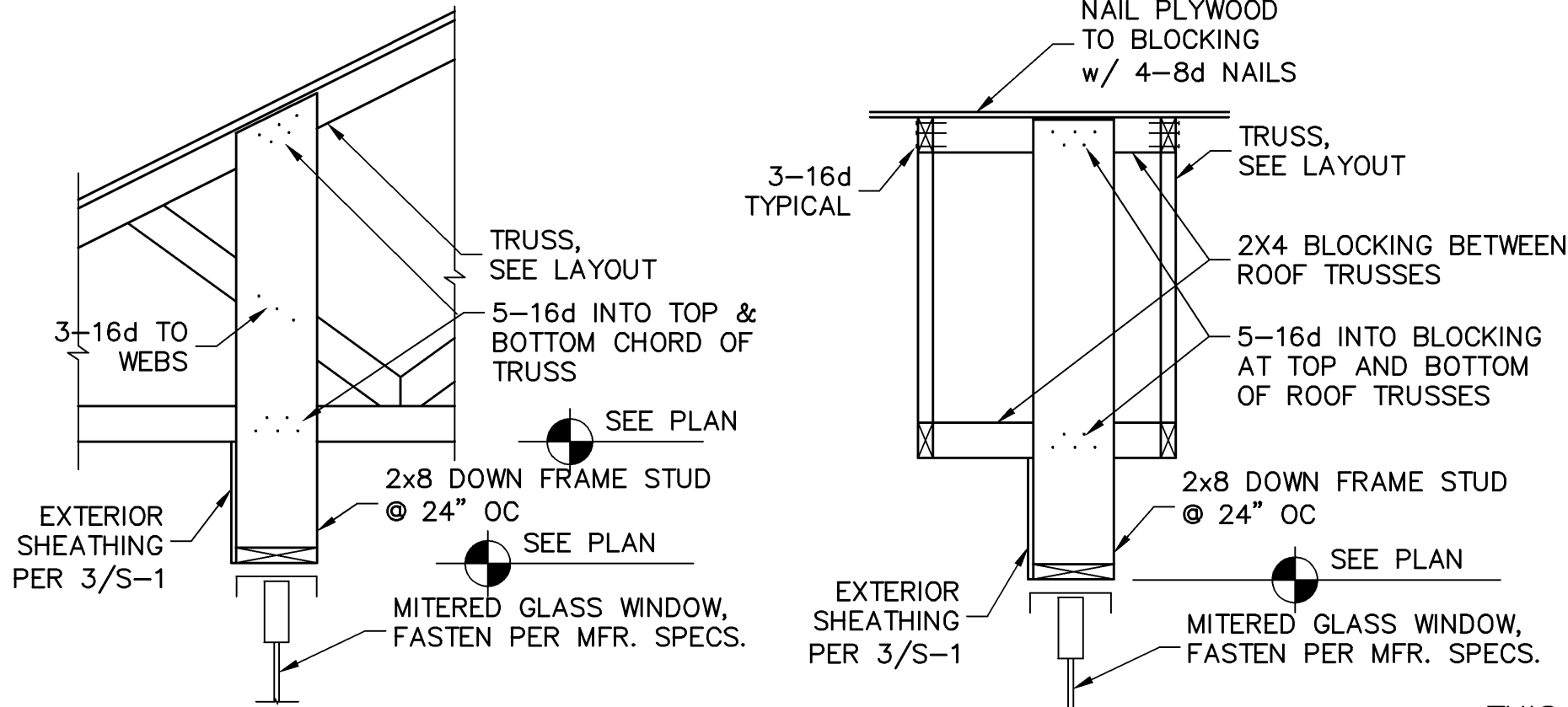
DESIGN/DRAWN  
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CHECKED  
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DATE  
12/31/15  
SCALE  
AS NOTED  
JOB NO.  
DR9093  
SHEET

S-1

SHEET 1 OF 2



1 **DOWN-FRAME HEADER**  
SCALE: 3/4" = 1'-0"



2 **SUSPENDED HEADER AT MITERED GLASS WINDOW**  
SCALE: 3/4" = 1'-0"

TRUSS PERPENDICULAR TO WINDOW      TRUSS PARALLEL TO WINDOW      THIS DETAIL ONLY USED FOR MITERED GLASS WINDOW OPTION

FOR RAYMOND TRUSSES, 170 MPH, ELEVATION E, EXTENDED LANAI, RBS # 12010249M11, DATED: 05/13/14, REVISED: 07/08/15

| REVISIONS | BY |
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