

2007 OHIO ASPHALT PAVING CONFERENCE

Pavement Friction Investigation

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Jeffrey, S. Kuttesch., "Quantifying the Relationship between Skid Resistance and Wet Weather Accidents for Virginia Data". Thesis (2004)

Skid resistance is statistically significant factor in explaining the wet accident rate.

Skid Number Requirements

Institution	FN	Speed (mph)/Type of Tire
FDOT Safety Improvement Program Manual	=>= 35 =>= 30	> 45/(Ribbed)< 45/(Ribbed)
FDOT Friction Testing and Action Program	- >= 35	REGARDLESS/(Ribbed)
OKDOT	- >= 35	REGARDLESS/(Ribbed)
NYDOT	□ >= 32	- 40/(Ribbed)
INDOT	- >= 20	40/(Smooth)
NCHRP-37	□ >= 37	■ REGARDLESS/(Ribbed)

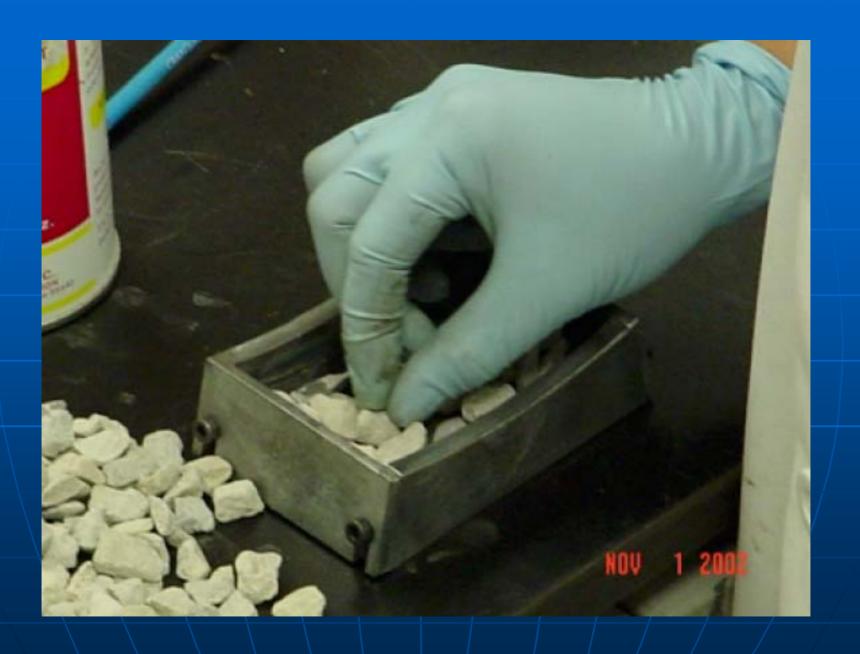
ODOT Commissioned Research

Author	Concerns
Liang and Chyi (2000)	 Polishing and friction characteristics of aggregates produced in Ohio
Liang (2003)	 Blending proportions of high skid and low skid aggregate
Liang (2005)	 Current research

Polishing and friction characteristics of aggregates produced in Ohio (2000)

20 Aggregate Sources in Ohio

- Accelerated Polishing
- Friction Measurement
- Petrographic Analysis
- Acid Insoluble Residue (AIR) Test
- Chemical Analysis (ODOT)
- Soundness (ODOT)











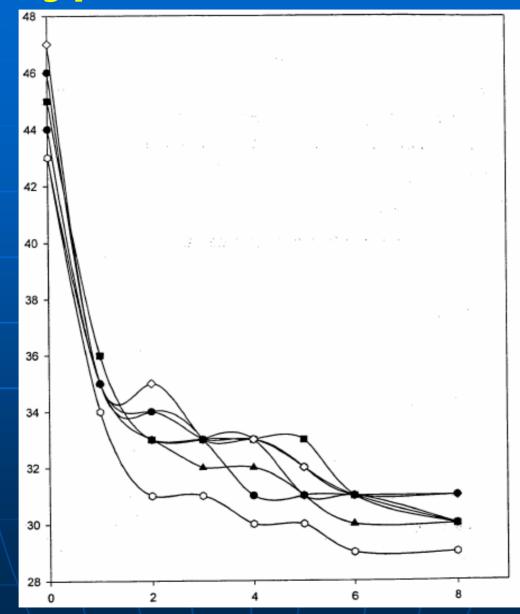








Typical Test Results



POLISH NUMBER (BPN)

TIME (HRS)

Acid Insoluble Residue (AIR) (ASTM D 3042)

Non-carbonate (Insoluble Residue) in Aggregates

The Higher the Amount of Acid Insoluble Residue



The Higher is the Skid Resistance

Soundness (ASTM C 88-90)

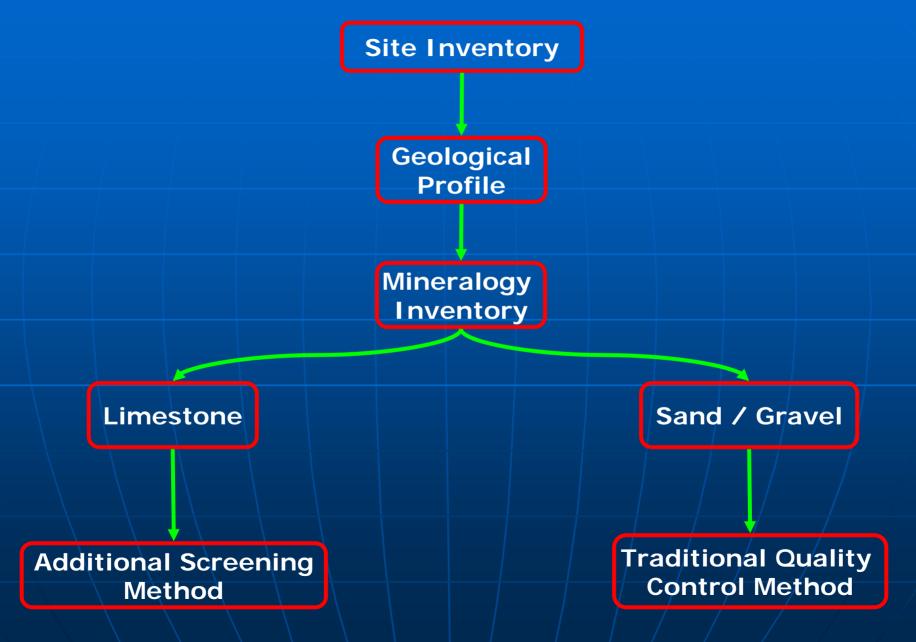
Tests the Pavement Aggregate Subjected to Weathering Changes

Good Polish Number Aggregate



Lower Soundness Loss Value

Aggregate Screening Method



Limestone

Aggregate Type

Gravel

Lab. Tests

Traditional Quality
Control Method

Acc. Polishing Tests

Chemical Analysis

AIR Tests

Detailed Petrography

Mineralogy Image Analyzer

Carbonate or non-Carbonate



Selection of Aggregate
Could Involve One or more
Selection Criteria

Criteria II
Calcite content 60-70%
Dolomite content 20-30%

Criteria I
TXDOT Recommendations
ADT PN
750-2000 28
2000-5000 30
5000-Above 32

Criteria III ALDOT Rec	_ commendations
BPN	Max. % of
	Carbonate Stone
<=25	30
26-28	35/
29-31	40
32-34	45
>=35	50

2. Non-Carbonate Aggregate

Selection of Aggregate
Could Involve One or more
Selection Criteria

Criteria I

TxDOT Recommendations

ADT

PN

750-2000

28

2000-5000

30

5000-Above

32

Criteria II

NYDOT Recommendations

If ADT < 3000, THEN AIR% <=15

If ADT > 3000, THEN AIR% > = 15

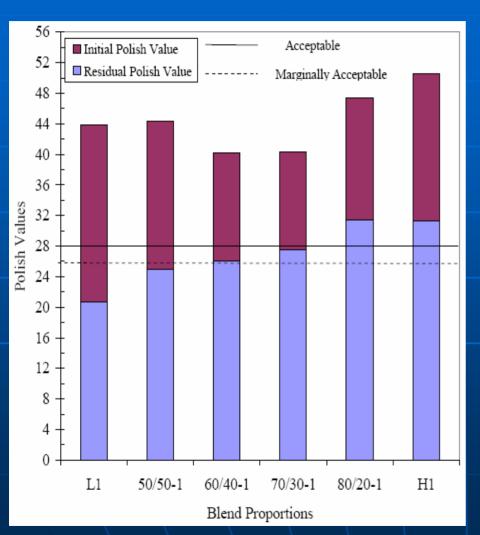
Blending proportions of high skid and low skid aggregate (2003)

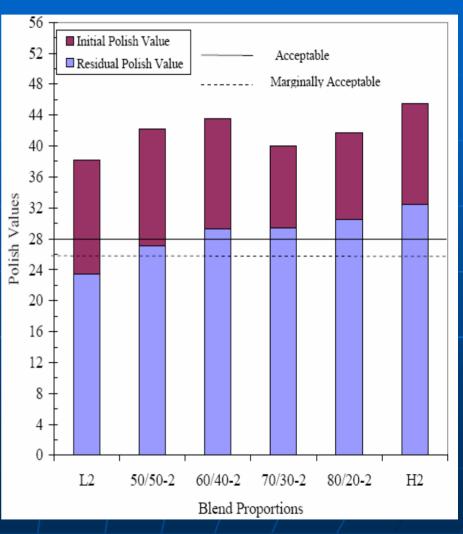
H = High Residual Friction

L = Low Residual Friction

Lab study to find optimum proportion

Lab Test Results Initial & Residual PV for Blends 1 & 2

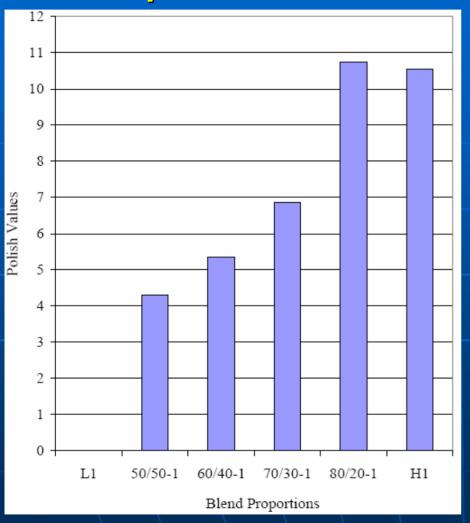


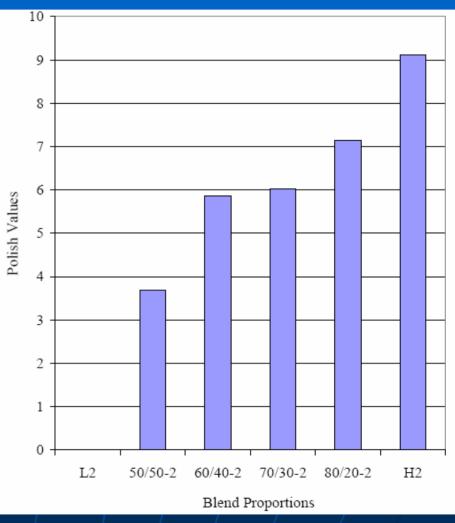


Blend 1

Blend 2

Lab Test Results Improvement in PV of Low Skid Resistant





Blend 1

Blend 2

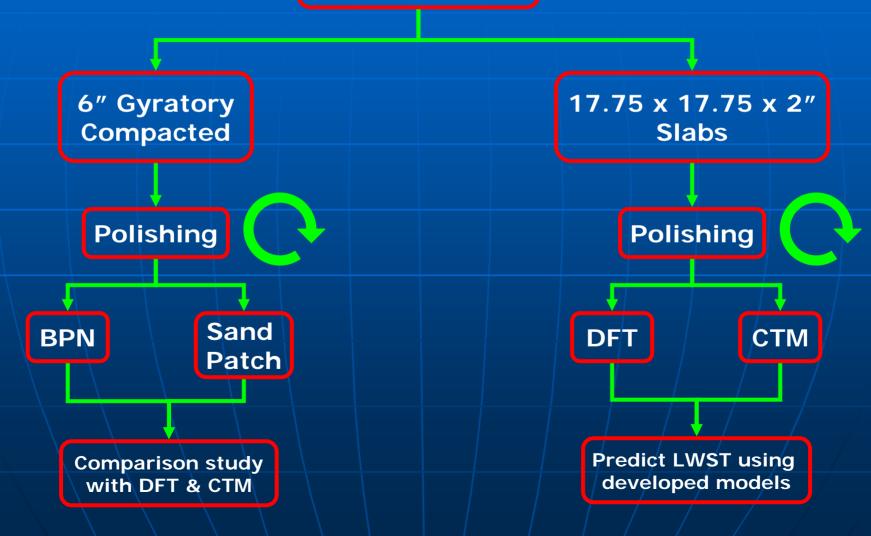
Objectives of Current Research

Develop new accelerated polishing equipment for compacted HMA specimens.

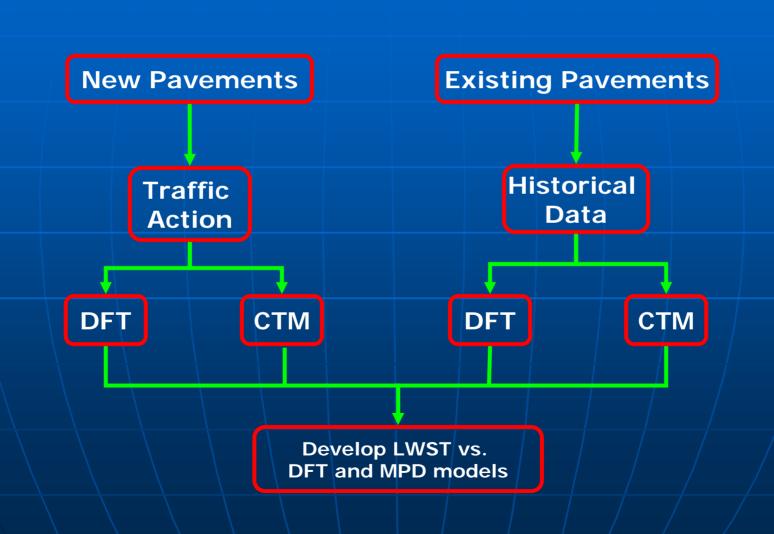
 Develop a complete test protocol and Recommend specifications for the new test methods.

Test Sequence for Laboratory Prepared HMA Specimens

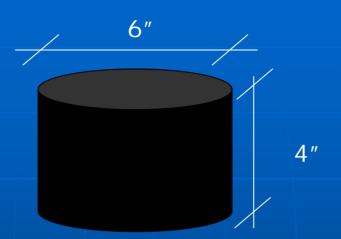
Superpave HMA Mix Design



Test Sequence for Field HMA Pavements



Research Equipment



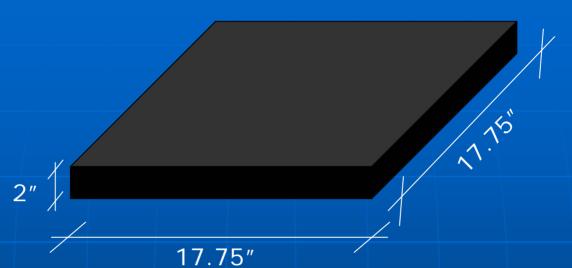






Sand Patch Method

Research Equipment



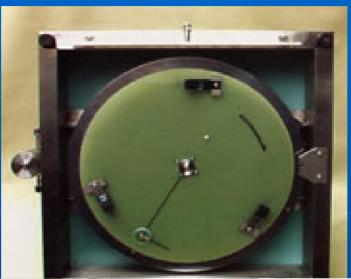


Dynamic Friction Tester



Circular Texture Meter

D.F.Tester



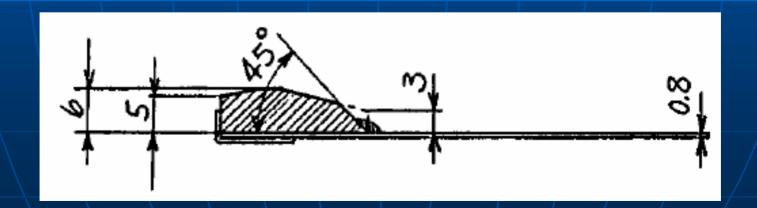




D.F.Tester

Sliders:

- 1. 0.25" x 0.63" x 0.79"
- 2. Synthetic rubber specified in ASTM E 501
- 3. Contact pressure = 21.5 psi
- 4. Hardness = 58



Circular Texture Meter (C.T.Meter)

Laser displacement sensor:

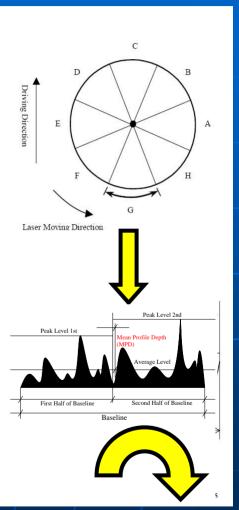
1. Spot size = 70 μm over a range of 65 to 90 mm



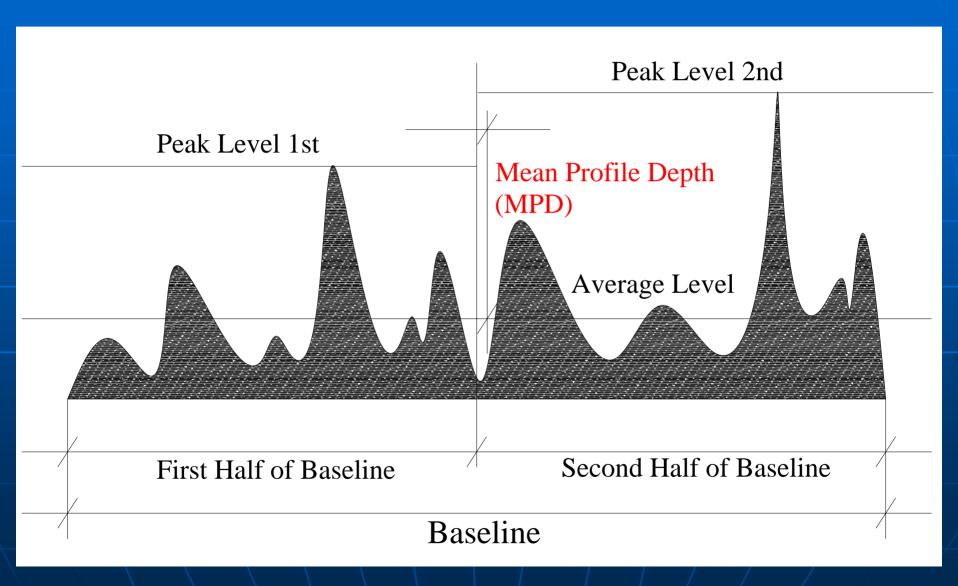
Picture-1 General View of Circular Texture Meter (CTM)



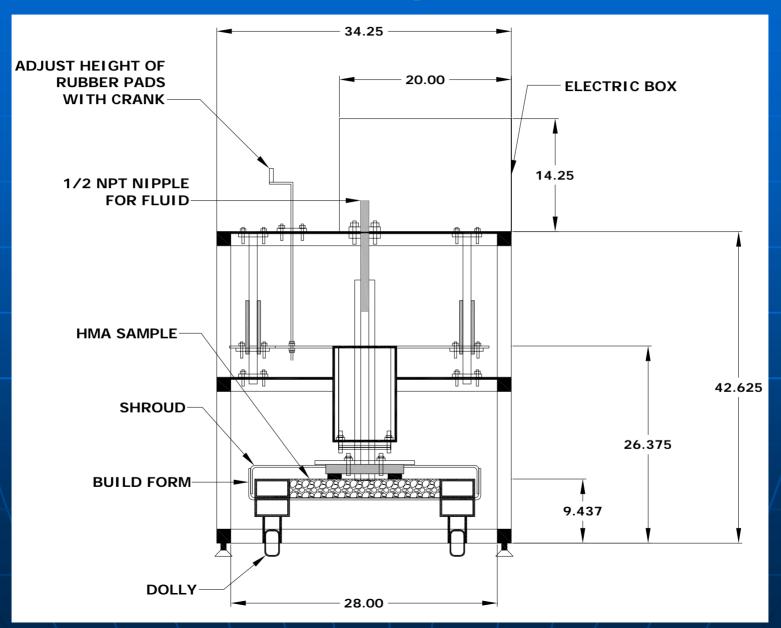
Picture-2 Laser Displacement Sensor of CTM



MPD Determination



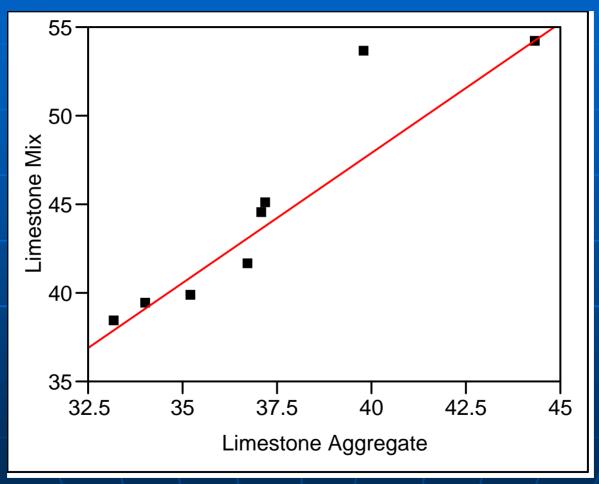
Rubber-Shoe Asphalt Polisher



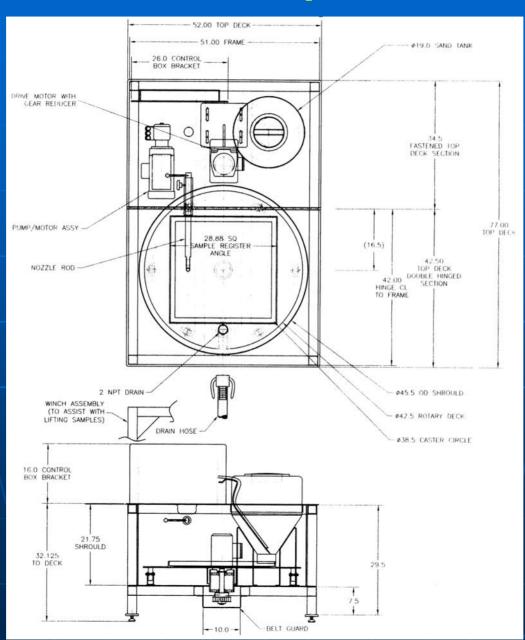
Rubber-Shoe Asphalt Polisher



Correlation between HMA and Aggregate Friction



Water-Pressure Asphalt Polisher



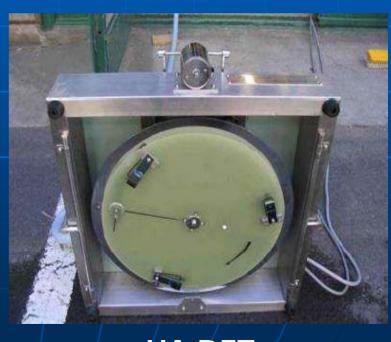
Water-Pressure Asphalt Polisher



Field Correlation Study



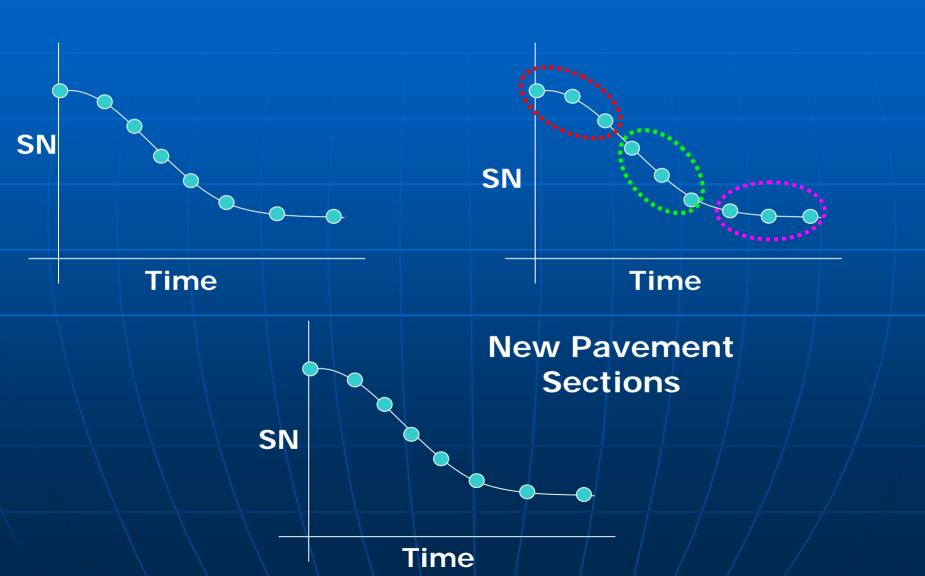
ODOT LWST



UA DFT



Existing Pavement Sections



Existing Pavement Sections



- 64 Friction data points.
- 64 Texture data points.

Existing Pavement Sections

Polish Level	Stockpile	District (County)	Roadway	Route (Section)	Project	# of Data Points
Possible high Polish (Gravel)	Chesterhill @ Stockport (Shelly)	10 (Washington)	2-Lane	7(37.3- 39.0)	99-98	8
Possible medium Polish (Limestone)	Hanson (Sandusky Crushed) @ Parkertown	3 (Huron)	2-Lane	250(3.55 -5.11)	401-00	6
Possible medium Polish (Dolomite)	Stoneco @ Maumee	2 (Wood)	4-Lane	25(15.68 -22)	22-03	40
Possible low Polish (Gravel)	Martin Marietta @ Apple Grove	11 (Harrison)	2-Lane	250(22.5 -25.5)	460-04	10

Existing Pavement Sections

District	Date	Temp. (°F)	
(County)	LWST	LWST	
	DFT	DFT	
10 (Washington)	09/20/60	60	
	10/25/06	51	
3 (Huron)	08/15/06	75	
	10/30/06	63	
2 (Wood)	10/08/06	70	
	10/18/06	64	
11 (Harrison)	5/24/06	55/	
	11/01/06	50	











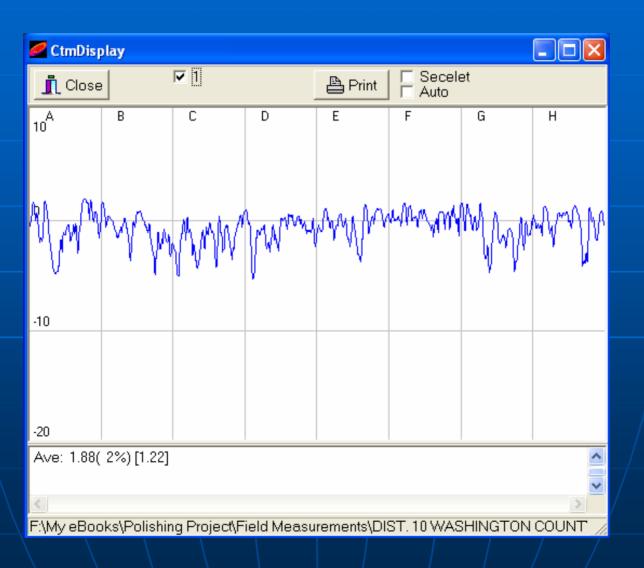




Sample Field Results (DFT)

Data Display			× X
Project Name		Measurement Location	
Measurement Site		Pavement Surface Type	
Weather		Moving Average	10 Frequency 1
Date	2006 / 10 / 25 Time 12 : 47	Operator	
Memo			
	J		
	Measurement Result	File Nam	ne
(1)	Sir	noothed Data	Number Average Run
(u) 1.0 0.9		1	out of 1 Runs 2
0.8		Coefficie	ent of Friction at
0.7 0.6		15 km/l	1 30 km/h 45 km/h 60 km/h 0
0.5 0.4		0.625	0.619 0.617 0.545 0.662
0.3 0.2			
0.1			
0.0 0 10	20 30 40 50 60 70 80	90 100	
		(km/h)	
Scale: S1 S2 S	3 Graph Setup < > Smoo	thing Load	Save Print Menu

Sample Field Results (CTM)



Simple Linear Regression Models

Correlation Between	Model	R ² (%)
LWST vs. DFT Lime. + Gravel	LWST = -14.49194 + 1.3941664 DFT	70.1
LWST vs. DFT Lime.	LWST = -24.98032 + 1.6529471 DFT	64
LWST vs. DFT Gravel	LWST = -19.40053 + 1.4582797 DFT	76.1
LWST vs. MPD	LWST = 43.033601 + 8.7581079 MPD	22.7

Transformed Models

Correlation Between	Model	R ² (%)
LWST vs. DFT Lime. + Gravel	Sqrt(LWST) = 2.2667266 + 0.1034807 DFT	70.5
LWST vs. DFT Lime.	Recip(LWST) = 0.0576172 - 0.0008248 DFT	69.3
LWST vs. DFT Gravel	Square(LWST) = -4813.828 + 154.20911 DFT	78.7
LWST vs. MPD	Square(LWST) = 967.37705 + 1827.2516 Sqrt(MPD)	25

Multiple Linear Regression Models using DFT & CTM

Model	R ² (%)	(R ²) _a (%)
LWST = -17.214+1.4943 DFT-2.438 MPD	71.1	70.2

IFI Determination

- Permanent International Association of road Conference (PIARC) Outcome
- Friction Measurement & Macrotexture Measurement

$$S_p = a + b \times T_x^{(mm)}$$
 Measured using CTM Measured using DFT
$$\frac{S-60}{S_p} + C \times T_x^{(mm)}$$

IFI is then reported as (F60,S_p)

ASTM E 1960

$$S_p = a + b \times T_x^{(mm)}$$

Tx Estimate	a	b
MPD (CTM)	14.2	89.7
MTD (Sand Patch)	-11.6	113.6

$$F60 = A + B \times FRS \times e^{\frac{S-60}{S_p}} + C \times T_x^{(mm)}$$

FRS Estimate	А	В	С
FN40R	-0.023	0.607	0.098
FN40S	0.045	0.925	0
DFT @ 40 mph	-0.034	0.771	0
DFT @ 12.5 mph	0.081	0.732	0

Multiple Linear Regression Models using IFI Parameters

Model	R ² (%)	(R ²) _a (%)
LWST = -18.542+1.8193 F600022 SC	71.2	70.2

Conclusions

 Aggregate screening methods for identifying high polishing/low skid resistance aggregates with acceptance criteria have been developed.

Blending has proven effective to increase residual friction values.

Conclusions

- Two prototype accelerated HMA polishing equipment have been developed.
- Aggregate is key controlling factor to HMA friction.

SN by LWST is significantly correlated with SN by DFT.

Conclusions

- Predictive equation for LWST measured SN is developed based on IFI parameters (F60, SC).
- Predictive equation for LWST measured SN is developed based on DFT & CTM measured values.

The last two findings enable us to link lab results with field data, thus allowing the establishment of acceptance criteria based on lab tests.



Thank You